

WATER MASTER PLAN AND CAPITAL IMPROVEMENT PLAN UPDATE

Valley of the Moon Water District El Verano, CA

15 April 2025 EKI C40120.00

EKI ENVIRONMENT & WATER, INC.



Water Master Plan and Capital Improvement Plan Update

Valley of the Moon Water District

15 April 2025

Prepared for:

Valley of the Moon Water District 19039 Bay Street El Verano, CA 95433-0280

Prepared by:

EKI Environment & Water, Inc. 2001 Junipero Serra Blvd., Suite 300 Daly City, California 94014 (650) 292-9100 www.ekiconsult.com EKI C40120.00

suther Art



Jonathan Sutter, P.E. Project Manager/Supervising Engineer

Un-la

Chris Pittner, P.E. Project Engineer



Oeno Juging

Yuqing Gao, EIT Project Engineer

Water Master Plan and Capital Improvement Plan Update

Valley of the Moon Water District

TABLE OF CONTENTS

ES	5	EX	(ECUTIVE SUMMARY ES	·1
1		IN	ITRODUCTION1	·1
	1.1		Project Scope1	·1
	1.2		Previous Evaluations and Planning Studies1	·1
	1.3		Report Organization1	·2
2		BA	ACKGROUND2-	·1
	2.1		Site Location and History2-	·1
	2.2		Service Area Climate2-	.3
	2.3		Number of Service Connections2	.3
	2.4		Service Area Demographics2-	-5
	2.5		Current and Projected Population2-	-5
	2.6		Anticipated Future Development2	-5
3		EX	(ISTING WATER SYSTEM FACILITIES	·1
	3.1		Water Supply Facilities	·1
	3.	1.1	SCWA Supply and Transmission System	-2
	3.2		Groundwater Supply	-3
	3.	2.1		
	3.	2.2		
	3.3		Emergency Interties	
	3.4		Water Distribution System Pressure Zones and Facilities	
	-	4.1		
	-	4.2 4.3		
	-	4.4		
	3.	4.5		
	3.	4.6	5 SCADA System	.9
4		EX	KISTING AND FUTURE WATER DEMANDS AND FIRE FLOW REQUIREMENTS4-	·1
	4.1		Current and Historical Water Production and Consumption4	·1
	4.2		Water Demand Projections4	·2
	4.	2.1		
		2.2		
	4.3		Peak Demands4-1	
	4.4		Fire Flow Requirements4-1	.5
5		W	ATER SUPPLY AND STORAGE CAPACITY EVALUATION	·1
	5.1		Storage and Supply Capacity Criteria5	·1



	5.1.1		
	5.1.2	Water Storage Capacity Requirements	5-2
	5.2	Supply and Pumping Evaluation	5-3
	5.3	Storage Evaluation	5-6
	5.4	Summary of Recommended Supply and Storage Facility Improvements	5-8
6	v	ATER DISTRIBUTION SYSTEM PERFORMANCE AND SIZING CRITERIA	6-1
	6.1	Distribution System Pressures	6-1
	6.2	Water Transmission and Distribution Pipeline Sizing Criteria	6-1
	6.3	Velocity Criteria	6-1
	6.4	Headloss Criterion	
7	w	ATER DISTRIBUTION SYSTEM MODELING EVALUATION	7-1
	7.1	Modeling Scenarios	
	7.2	Modeling Approach	7-3
	7.2.1	PHD Simulations	7-3
	7.2.2	MDD + FF Simulations	7-3
	7.3	Existing Scenario Evaluation	7-4
	7.3.1	PHD Results	7-4
	7.3.2	MDD Plus Fire Flow Results	7-4
	7.4	Future Scenario Evaluation	7-7
	7.4.1		
	7.4.2	MDD Plus Fire Flow Results	7-7
	7.5	Recommended Distribution System Capacity Improvements	7-7
	7.6	District Metered Areas	7-7
8	R	COMMENDED CAPITAL IMPROVEMENT PROGRAM	
	8.1	CIP Costs	8-1
	8.1.1	Pipeline Project Costs	8-1
	8.1.2	Treated Water Storage Tank Costs	8-2
	8.1.3		
	8.1.4		
	8.1.5		
	8.1.6	Recommended Priorities	8-3
	8.2	Capital Improvement Projects	8-4
9	R	FERENCES	9-1



TABLES

Table ES-1. Projected Existing and Future Annual Demands by Pressure Zone	ES-3
Table ES-2. Summary of Recommended Water system Capital Improvement Projects	ES-6
Table 2-1. Climate Characteristics	2-3
Table 2-2. Number of Current and Historical Potable Water Services by Customer Category	2-4
Table 2-3. Population Projections	2-5
Table 3-1. SCWA Turnout Information	3-7
Table 3-2. Groundwater Well Information	3-11
Table 3-3. Pressure Zone Information	3-12
Table 3-4. Current and Historical Potable Water Services by Pressure Zone and Customer Class .	3-13
Table 3-5. PRV Station Information	3-14
Table 3-6. Water Storage Facility Information	3-15
Table 3-7. Booster Pump Station Information	3-16
Table 3-8. Pipeline Lengths by Diameter and Material	3-17
Table 4-1. Current and Historical Potable Water Use and Population	4-3
Table 4-2. Current and Historical Potable Water Production by Source	4-5
Table 4-3. Current and Historical Potable Water Use by Customer Category	4-7
Table 4-4. Current and Historical Potable Water Use by Pressure Zone	4-9
Table 4-5. Current and Historical Potable Water Use by Pressure Zone and Customer Category	4-11
Table 4-6. Projected Water Demands for Anticipated New Development	4-13
Table 4-7. Projected Existing and Future Annual Demands by Pressure Zone	4-14
Table 4-8. Sources of Inflow and Outflow and Storage by Pressure Zone	4-16
Table 4-9. Summary of Peaking Factors by Pressure Zone	4-17
Table 4-10. Projected Existing and Future Peak Demands by Pressure Zone	4-18
Table 5-1. Water Supply Capacity Requirements by Pressure Zone	5-5
Table 5-2. Water Storage Capacity Requirements by Pressure Zone	5-7
Table 5-3. Water Supply Capacity Requirements by Pressure Zone with Recommended Improve	ments5-9
Table 5-4. Water Storage Capacity Requirements by Pressure Zone with Recommended Improve	eme.5-10
Table 6-1. Summary of Recommended Potable Water System Performance and Operational Crit	teria6-3
Table 8-1. Unit Construction Cost for Pipeline Projects	8-2
Table 8-2. Miscellaneous Costs for Pipeline Projects	8-2
Table 8-3. Miscellaneous Costs	8-3
Table 8-4. Summary of Recommended Water System Capital Improvement Projects	8-6



FIGURES

Figure ES-1. Recommended Capacity-Related Water System Improvements	ES-5
Figure 2-1. District Service Area	2-2
Figure 2-2. Current (FY 2024) Potable Water Services by Customer Category	2-4
Figure 3-1. Existing Water System Facilities	
Figure 3-2. Existing Water System Hydraulic Profile	3-5
Figure 3-3. SCWA Service Area and Water Transmission System Facilities	
Figure 3-4. Summary of Local and Regional Groundwater Management	
Figure 3-5. Existing Water Main Sizes	
Figure 3-6. Existing Water Main Materials	
Figure 4-1. Current and Historical Potable Water Use and Population	4-4
Figure 4-2. Current and Historical Per Capita Potable Water Use	4-4
Figure 4-3. Current and Historical Potable Water Production by Source	
Figure 4-4. Annual Water Use by Customer Category: FY 2010 - FY 2024	4-7
Figure 4-5. Percentage of Total Water Use by Customer Category: FY 2010 - FY 2024	4-8
Figure 4-6. Annual Water Use by Pressure Zone: FY 2010 - FY 2024	4-10
Figure 4-7. Percentage of Total Water Use by Pressure Zone: FY 2010 - FY 2024	4-10
Figure 4-8. Required Fire Flow Availability Map	4-19
Figure 7-1. Projects in Development Included in Future Modeling Scenario	7-2
Figure 7-2. Existing Water System Performance Evaluation – Peak Hour Demand – System	Pressures7-5
Figure 7-3. Existing Water System Performance Evaluation – Max Day Demands – Available	e Fire Flow .7-6
Figure 7-4. Future Water System Performance Evaluation – Peak Hour Demand – System P	ressures7-9
Figure 7-5. Future Water System Performance Evaluation – Max Day Demands – Available	Fire Flow.7-10
Figure 7-6. Future Water System Performance Evaluation – PHD– System Pressures with C	IPS7-11
Figure 7-7. Future Water System Performance Evaluation – Max Day Demands – Available	Fire Flow.7-12
Figure 8-1. Recommended Capacity-Related Water System Improvements	8-5

APPENDICES

Appendix A – CIP Project Detail Sheets



ABBREVIATIONS AND ACRONYMS

ACP	asbestos cement pipe
ADD	average day demands
AF	acre feet
AFY	acre feet per year
AMI	advanced metering infrastructure
ASR	Aquifer Storage and Recovery
BPS	Booster Pump Station
CCI	Construction Cost Index
CIP	Capital Improvement Program, Capital Improvement Project, or cast-iron pipe
DIP	ductile iron pipe
District	Valley of the Moon Water District
DMA	District Metered Areas
DSS	Decision Support System
du	dwelling unit
DWR	California Department of Water Resources
ea	each
EKI	EKI Environment & Water
EL	elevation
ENR	Engineering News Record
ES	Executive Summary
ETo	reference evapotranspiration
°F	degrees Fahrenheit
FF	Fire Flow
fps	feet per second
ft	feet
ft/k-ft	feet per thousand feet
FY	fiscal year
gal	gallons
General Plan	2005 Sonoma County General Plan
GIS	geographic information system
GPCD	gallons per capita per day
gpd	gallons per day
gpm	gallons per minute
GSA	Groundwater Sustainability Agency
GSP	Groundwater Sustainability Plan
HDPE	high-density polyethylene
hp	horsepower
In	inch



JPA	Joint Exercise of Powers Agreement
LAFCO	Sonoma Local Agency Formation Commission
LF	linear feet
MDD	Max Day Demands
MFR	Multi-Family Residence
MG	Million Gallons
MGD	million gallons per day
MMD	maximum month demand
NAD83	North American Vertical Datum 1983
NAVD 88	North American Vertical Datum of 1988
OPC	Opinion of Probable Cost
PHD	Peak Hour Demands
PRV	pressure reducing valve
psi	pounds per square inch
PVC	polyvinyl chloride
PZ	pressure zone
Restructured Agree	ement Restructured Agreement for Water Supply
SBx7-7	2020 Senate Bill x7-7
SCADA	Supervisory Control and Data Acquisition
SCWA	Sonoma County Water Agency
SCWA SDC	Sonoma County Water Agency Sonoma Development Center
SDC	Sonoma Development Center
SDC sf	Sonoma Development Center square feet
SDC sf SFR	Sonoma Development Center square feet Single Family Residence
SDC sf SFR SGMA	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014
SDC sf SFR SGMA SVMWC	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014 Sobre Vista Mutual Water Company
SDC sf SFR SGMA SVMWC SWRCB	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014 Sobre Vista Mutual Water Company State Water Resources Control Board
SDC sf SFR SGMA SVMWC SWRCB Title 22	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014 Sobre Vista Mutual Water Company State Water Resources Control Board California Code of Regulations, Title 22
SDC sf SFR SGMA SVMWC SWRCB Title 22 UNK	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014 Sobre Vista Mutual Water Company State Water Resources Control Board California Code of Regulations, Title 22 unknown
SDC sf SFR SGMA SVMWC SWRCB Title 22 UNK UWMP	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014 Sobre Vista Mutual Water Company State Water Resources Control Board California Code of Regulations, Title 22 unknown Urban Water Management Plan
SDC sf SFR SGMA SVMWC SWRCB Title 22 UNK UWMP USACE	Sonoma Development Center square feet Single Family Residence Sustainable Groundwater Management Act of 2014 Sobre Vista Mutual Water Company State Water Resources Control Board California Code of Regulations, Title 22 unknown Urban Water Management Plan U.S. Army Corps of Engineers



EXECUTIVE SUMMARY

On behalf of the Valley of the Moon Water District (District), EKI Environment & Water, Inc. (EKI) has prepared this Water Master Plan (WMP) and Capital Improvement Plan (CIP) Update. The District's WMP was last updated in 2019 (EKI, 2019), and since then the District has addressed many of the capital improvement projects recommended in that plan. The details for several other projects have been modified in the interim, and the District has identified new condition-related improvement needs. In addition, several planning assumptions from the 2019 WMP have changed, including the existing and projected demands and storage evaluation criteria for the Glen Ellen Area (Zone 1F). Lastly, the District has identified three new locations to install new pressure zones or district metered areas (DMAs) to better manage high and low pressures.

District's Existing Water Infrastructure

The District's water system facilities have been updated based on completed and in-progress CIP projects, many of which were identified in the 2019 WMP, including:

- The addition of the Pedroncelli and Craig groundwater wells
- The installation of the new Saddle Tank
- Modifications to the District's pressure reducing valve (PRV) settings
- Installation of approximately 7,715 linear feet of water mains based on approximately 37 individual as-builts listed as project groups below:
 - EST 2947 The Walnut Ave, Oak St. & Penny Ln. Water Main Replacement Project
 - <u>EST 2958</u> Aqua Caliente Road, Arnold Drive, and Summer Meadow Lane Main Replacement Project (including a new normally closed cross connection between PZ1 and PZ1B)
 - EST 2967 The Boyes Blvd. Bridge Pipeline Replacement Project
 - EST 2983 The Pedroncelli and Craig Well installations
 - <u>EST 2984</u> The Gibson St., Riddle Rd Easement, Sobre Vista (near Lake Josephine), Brookview & Lomita Steel Water Main Replacements
 - <u>EST 2993</u> The Fetters Ave., Malek Rd., Depot Rd., Sobre Vista Dr., and Wake Robin Rd. Main and Service Line Replacement Project
 - EST 2996 The Arnold Dr Main Replacement Project
 - <u>EST 3021</u> The Waterline Improvement Project on Chestnut Rd to Chestnut Tank
 - EST 3034 to EST 3040 Various service line, and hydrant installations
- Upcoming capital improvement projects currently in design or construction, including:
 - Altimira Fire Flow Improvement Project
 - Verano Hotel Frontage Public Water Main Improvements;
 - 18661 Lomita Avenue New Water Main Project
 - Boyes Food Center Mixed Use Development Water Main Improvement Project



Existing and Future Water Demands

Potable water use has generally decreased over the past 20 years, although significant variations have occurred from year to year and are associated with changing hydrologic and economic conditions. Following the 2013-2016 drought, the District's total and per capita water use increased slightly between fiscal year (FY) 2017 and FY 2021, reflecting a partial drought rebound. However, total demand decreased in FY 2022 through FY 2024 in response to the recent drought.

Based on a review of demand data and discussions with the District, the water demand data from FY 2021, prior to the recent drought, was assumed to be representative of existing demands. Total existing demand was estimated to be 2,853 acre-feet per year (AFY) for planning and modeling purposes, equal to the total average FY 2021 billing data plus the maximum construction water and water loss from the last five years (FY 2020 - FY 2024). To account for 32 new accounts added since FY 2021, demand was added for these accounts based on land use specific FY 2021 demand factors.

The total projected future demand is estimated to be 3,477 AFY in FY 2045, consistent with demand projections presented in the District's 2020 Urban Water Management Plan (UWMP). Future demands for planned development projects were spatially allocated based on their location and land uses, and the remaining increase in demand was spread proportionally across the District. Existing and future projected demands are summarized in Table ES-1 by pressure zone.

Water System Supplies

The District, along with seven other cities and special districts in Sonoma County and Marin County, has a water supply agreement with Sonoma County Water Agency (SCWA). The majority of the District's water supply comes from SCWA purchases and is delivered through the Sonoma Aqueduct (approximately 79% on average over the past six years). The District's remaining water is supplied by eight (8) groundwater wells that are owned (or leased) and operated by the District.

Supply and Storage Capacity Assessment

EKI recommends firm supply capacity requirements for each pressure zone met through the combination of SCWA turnouts, groundwater wells, and booster pump stations to meet the applicable combination of max day demands, peak hour demands, fire flow requirements, and/or fire storage refilling requirements based on the available storage in each zone. EKI recommends continuing to use water supply and storage criteria that were updated in the *Evaluation of Storage and Supply Requirements for Glen Ellen* (EKI, 2021b).

The supply assessment indicates that Pressure Zones 2E, 3E, 2B, 3D, and 1F have supply capacity deficits under existing and future conditions. To meet the supply criteria for each zone, EKI recommends the following improvements:

- Installation of a new 450 gallons per minute (gpm) booster pump station (BPS) to deliver supply from Pressure Zone 1B to Pressure Zone 1F;
- Installation of dedicated 1,000 gpm fire pumps at Donald BPS and Chestnut BPS; and
- Upgrades to the Sobre Vista Lower BPS as part of a consolidation of Pressure Zones 2E and 3E.



			Future Demands (AFY)
Pressure Zone	Existing Demands (AFY) (a)	Projected Infill Growth (b)	Planned New Development (c)	Total Future Demands (b) (d)
1	1,916	2,060	259	2,319
1A	224	241	-	241
1B	236	254	150	404
1F	257	277	-	277
2A	2.0	2	-	2.1
2B	20	21	-	21
2D	107	115	-	115
2E	0.29	0.31	-	0.31
3D	14	15	-	15
3E	48	52	-	52
4E	0.22	0.24	-	0.24
5E	5.3	6	-	5.7
SCWA	24.1	26	-	26.0
Total	2,853	3,042	409	3,477

Table ES-1. Projected Existing and Future Annual Demands by Pressure Zone

Notes:

- (a) Equals the total average FY 2021 billing data by pressure zone plus maximum construction water and the max water loss from the last 5 years (FY 2020 - FY 2024). To account for the 32 new accounts added since 2021, demand was added for each account based on land use specific FY 2021 demand factor.
- (b) Projected infill growth and total future demands for each pressure zone are proportionate to the pressure zones' existing demands.
- (c) Planned new developments include the proposed 810 W Agua Caliente Development, Verano Avenue Multi-Family Residential Development, and the Springs Specific Plan.
- (d) Total future demand of 3,477 AFY in 2045 per the District's 2020 UWMP.

Based on discussions with the District, we recommend installing the new Eldridge BPS and relocating the existing Eldridge PRV north of the Sonoma Developmental Center (SDC). This location would allow for surface water supplies from SDC to be pumped to Pressure Zone 1F if these facilities are annexed by the District in the future.

Based on the system-wide storage evaluation, a significant 3.87 million gallons (MG) storage surplus exists. With the District's current storage facilities and accounting for SCWA storage and groundwater supplies, existing emergency storage ranges from 3.5 to 4.2 days of average day demand (ADD) and future available emergency storage is projected to range between 3.2 to 3.7 days of ADD depending on the demand assumptions. This represents a robust volume of emergency storage that should allow for the District and SCWA under most scenarios to address any supply disruption for the storage is depleted.

Hydraulic Assessment

To evaluate distribution system performance against performance criteria, EKI conducted steady-state model simulations of (1) peak hour demand (PHD) and (2) maximum day demand (MDD) plus fire flow (FF) for both the Existing, Future, and Future with CIPs Scenarios. As a part of this modeling analysis, the District identified three areas in the system which could benefit from the installation of new pressure



regulating stations to reduce pressures. These three areas have been included in the Future with CIPs scenario to ensure no unintended effects occurred with the re-zoning of these portions of the system.

EKI modeled the remaining proposed improvements from 2019 under future demand conditions to confirm that the identified deficiencies had been addressed. Generally, the remaining projects identified in the 2019 WMP have been carried forward without any modifications, indicating that they are still appropriate solutions to the identified deficiencies. However, one project, the Agua Caliente Road Transmission Improvement, has been removed from the CIP list due to the proposed new pressure zone modifications in the Agua Caliente Knolls area, as discussed below in Section 7.6, which also addresses the fire flow deficiencies identified in this area.

Recommended Updates to the Capital Improvement Program

Figure ES-1 shows an overview of the recommended improvements. A summary of the recommended improvements, as well as the CIPs included in the District's existing 5-year CIP budget, are presented in Table ES-2. As shown in Table ES-2, the total OPC for the proposed CIP in December 2024 dollars is approximately \$27.0 million to \$34.9 million (depending on whether the pipeline projects will be constructed by the District or a construction contractor). It should be noted that the recommended CIP only identifies improvements at a master plan level and does not constitute a design of such improvements. Subsequent detailed design is required to determine the exact sizes and locations of these proposed improvements. The recommended CIP is discussed in more detail in Section 8.



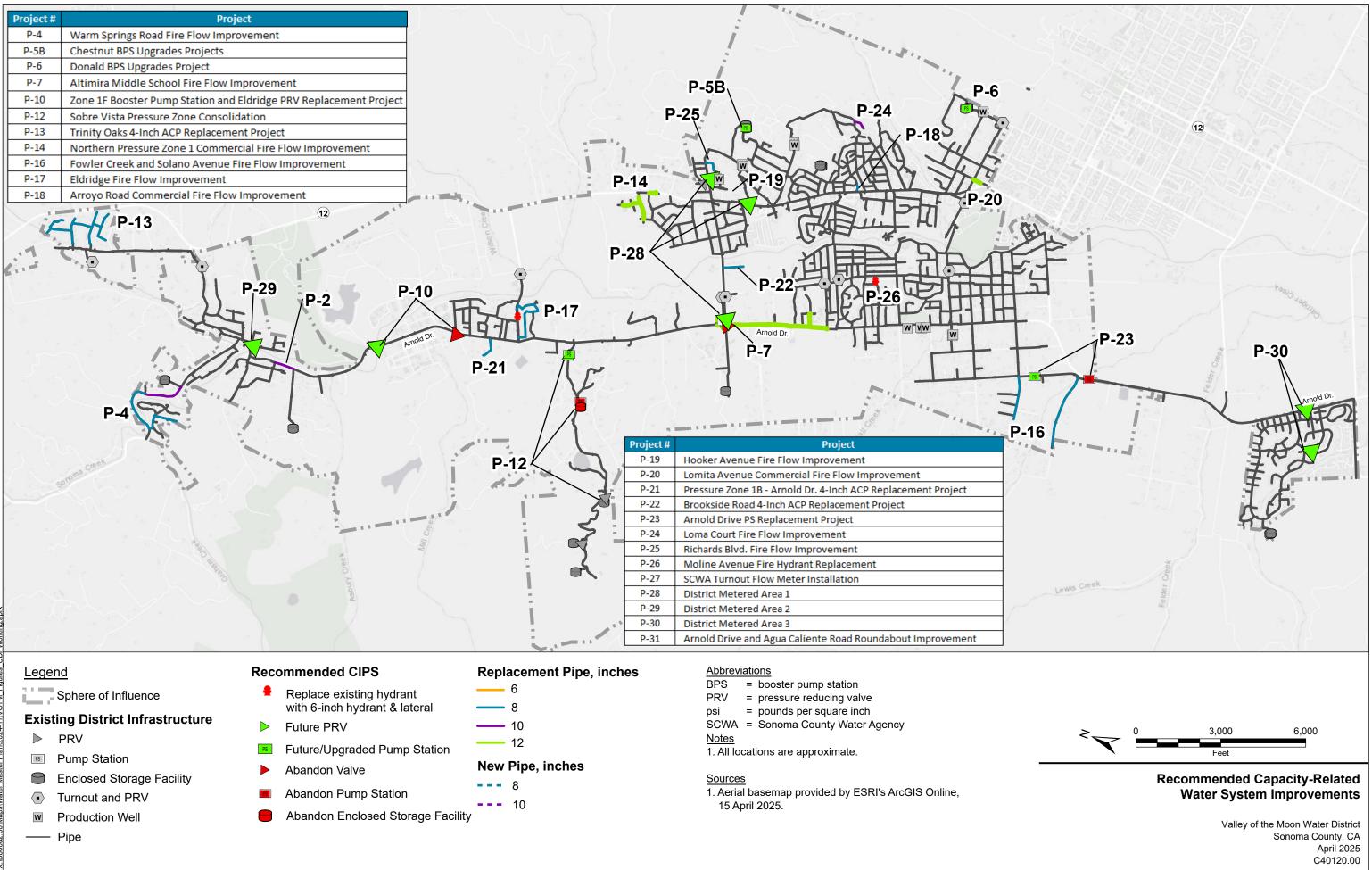


Figure ES-1

Table ES-2 Summary of Recommended Water System Capital Improvement Projects

	Project	Improvement Description		Recommended Pipe	Pipe Length	Total Pro (a)	oject OPC (b)
Project #	Project			Diameter (in)	(Linear Feet)	District Staff	External Contractor
Facilities a	and Maintenance Projects						
P-29	District Metered Area 2 (PZ-1G)	Install new 8-inch PRV station with flow metering at the intersection of Kearney Avenue and East Agua Caliente Road, running parallel to the existing zone separating closed valve, and new 12-inch PRV stations with flow metering (1) on West Agua Caliente Road east of the roundabout (2) on Highway 12 between Vailetti Drive and Sunnyside Avenue to create new pressure zone 1G in the Agua Caliente Knolls area.	1			\$670,000	\$920,000
P-27	SCWA Turnout Flow Meter Installation	Install flow meters at each of the SCWA turnout PRVs and integrate with SCADA system.	2			\$770,000	\$1,040,000
P-28	District Metered Area 1 (PZ-1H)	Install new 6-inch PRV station with flow metering at the corner of Arnold Drive and Carmel Ave and create new pressure zone in Glen Ellen.	2			\$230,000	\$330,000
P-30	District Metered Area 3 (PZ-1I)	Install new 6-inch PRV stations with flow metering on (1) Avenida Sebastiani between Via Colombard and Avenida Barbera and (2) on Arnold Drive between Mission Drive and Avenida Sebastiani, and close the valve on South Temelec Circle between Mission Drive and Herbazal Street to create new pressure zone 1I in the Temelec Area.	2			\$450,000	\$620,000
Pipeline P	rojects						
P-4	Warm Springs Road Fire Flow Improvement	Replace existing 6-inch PVC, ACP, and DIP water mains with new 8-inch and 10-inch PVC water mains, replace 47 existing service connections, and replace four existing fire hydrants.	1	<u> </u>	3,400 1,500	\$1,990,000	\$3,120,000
		Replace existing 6-inch and 8-inch PVC and ACP water mains with new 12-inch PVC water mains along Arnold Drive, replace existing 6-inch pipe with new 8 and 12-inch pipe adjacent to Altimira		10	50		
P-7	Altimira Middle School Fire Flow Improvement	Middle School, replace 15 existing service connections, and replace three existing fire hydrants. This project could be combined with P-31 for efficiency.	1	12	4,235	\$2,210,000	\$3,290,000
P-13	Trinity Oaks 4-Inch ACP Replacement Project	Replace existing 4-inch ACP water mains with new 8-inch PVC water mains, replace 49 existing service connections, and replace six existing fire hydrants in the Trinity Oaks area. District to coordinate with Fire Department to determine if additional hydrants are needed. These hydrants would be funded by the Fire Department.		8	6,000	\$2,280,000	\$3,550,000
P-17	Eldridge Fire Flow Improvement Replace the beneficient of the ACP water main on Madrone Avenue and reconnect services to existing 8-inch water mains. This project has been identified as high priority due to the condition of the ACP water mains in th zone.		1	8	3,900	\$1,540,000	\$2,470,000
P-31	Arnold Drive and Agua Caliente Road Roundabout Improvement	Replace existing 8-inch ACP water mains with new 12-inch PVC water mains and relocate the existing Hannah Lower PRV out of the center of the new roundabout. This project has been identified as high priority due to the safety concerns with operating this PRV. This project could be combined with P-7 for efficiency.	1	12	2,000	\$1,230,000	\$1,760,000
P-14A	Northern Pressure Zone 1 Commercial Fire Flow Improvement - La Grama	Replace existing 6-inch water mains with new 12-inch PVC water mains, replace 3 existing service connections, and replace three existing fire hydrants.	2	12	1,425	\$760,000	\$1,150,000
P-16	Fowler Creek and Solano Avenue Fire Flow Improvement	Replace existing 6-inch ACP water mains with new 8-inch PVC water mains, replace ten existing service connections, and replace five existing fire hydrants.	2	8	4,200	\$1,550,000	\$2,380,000
P-14B	Northern Pressure Zone 1 Commercial Fire Flow Improvement - HWY 12	Replace existing 8-inch ACP water mains with new 12-inch PVC water mains, replace one existing service connections, and replace one existing fire hydrants. Based on discussions with the District, the commercial areas along HWY 12 have been vacant in this area for an extended period. This project is only recommended if new development occurs here.	3	12	280	\$160,000	\$260,000



 Table ES-2 (cont.)

 Summary of Recommended Water System Capital Improvement Projects

Project #	Project	Improvement Description	Priority	Recommended Pipe Diameter	Pipe Length	Total Project OPC (a)(b)	
Troject #	Tojett		Thorky	(in)	(Linear Feet)	District Staff	External Contractor
Pipeline Pr	rojects						
P-18	Arroyo Road Commercial Fire Flow Improvement	Install new 8-inch PVC water main between Highway 12 and Madera Road along Arroyo Road.	3	8	200	\$80,000	\$130,000
P-19	Hooker Avenue Fire Flow Improvement	all new 8-inch PVC water main between Highway 12 and Hooker Ave.		8	550	\$200,000	\$300,000
P-20	Lomita Avenue Commercial Fire Flow Improvement	Replace existing 6-inch ACP water main with new 12-PVC water main along Lomita Avenue, replace two service connections, and replace one hydrant.	3	12	300	\$170,000	\$280,000
P-21	Pressure Zone 1B - Arnold Dr. 4-Inch ACP Replacement Project	Replace existing 4-inch ACP water main with new 8-inch PVC water main in Pressure Zone 1B west of Arnold Drive, and replace three existing service connections.	3	8	800	\$290,000	\$440,000
P-22	Brookside Road 4-Inch ACP Replacement Project	place existing 4-inch ACP water main with 8-inch PVC water main along Brookside Road and place eight existing service connections.		8	800	\$300,000	\$470,000
P-24	Loma Court Fire Flow Improvement	Replace existing 6-inch with new 8-inch PVC along Loma Court, replace 11 existing service connections, and replace one existing fire hydrant.		8	500	\$220,000	\$370,000
P-25	Richards Blvd. Fire Flow Improvement	Replace existing 6-inch ACP and DIP water main with 8-inch PVC water main along Richards Blvd, replace four existing service connections, and one existing hydrant.	3	8	300	\$130,000	\$240,000
Pump Stat	ions, Tanks, and Wells			•	•		
P-5B	Chestnut BPS Upgrades Projects	Replace existing Chestnut BPS with two (2) 100-gpm domestic pumps and one (1) 1,000 gpm fire pump at 60 ft total dynamic head (TDH).	1				\$2,600,000
P-6	Donald BPS Upgrades Project	Replace existing Donald BPS with two (2) 115-gpm domestic pumps and one (1) 1,000 gpm fire pump at 220 ft TDH.	1				\$2,600,000
P-10	Zone 1F Booster Pump Station and Eldridge PRV Replacement Project	Install new PRV and BPS with a firm capacity of 450 gpm at 275 ft TDH. Abandon existing Eldridge PRV.	2				\$2,130,000
P-12	Sobre Vista Pressure Zone Consolidation	eplace Lower Sobre Vista BPS with a firm capacity of 290 gpm at 270 ft TDH; demolish Lower obre Vista Tank and Upper Sobre Vista BPS; connect PZ-2E and 3E; install individual service PRVs in ormer PZ-2E area; Replace existing 8-inch ACP water mains with new 8-inch PVC water mains.					\$2,650,000
P-23	Arnold Drive PS Replacement Project	Install new BPS with a firm capacity of 500 gpm along Orange Avenue. Demolish existing Arnold Drive BPS.	3				\$1,800,000
		TOTAL	WATER DIST	RIBUTION SYSTEM IMPR	ROVEMENTS OPC (c)	\$27,010,000	\$34,900,000

Notes:

(a) Costs shown are presented in December 2024 dollars based on an ENR CCI of 15,400.54 (20-city average), with totals rounded to the nearest \$10,000.

(b) Costs for pipeline projects include construction contingency (25%), design (10%), construction management (5%), permitting (5%), and Project Implementation (5%). Costs for other projects (i.e. BPS installations) include construction contingency (30%), design (15%), construction management (5%), permitting (5%), and Project Implementation (5%).

(c) Total district constructed OPC includes contractor costs for pump station, tanks, wells, and other projects not anticipated to be constructed by the District.

eki environment & water

1 INTRODUCTION

On behalf of the Valley of the Moon Water District (District), EKI Environment & Water, Inc. (EKI) has prepared this Water Master Plan (WMP) and Capital Improvement Plan (CIP) Update. The District's WMP was last updated in 2019 (EKI, 2019), and since then the District has addressed many of the capital improvement projects identified in that plan. The details for several other projects have been modified in the interim, and the District has identified new condition-related improvement needs. In addition, several planning assumptions from the 2019 WMP have changed, including the existing and projected demands and storage evaluation criteria for the Glen Ellen Area (Zone 1F). Lastly, the District has identified three new locations to install new pressure zones or district metered areas (DMAs) to better manage high and low pressures.

The District has requested an update to the District's WMP and CIP that reprioritizes remaining and newly identified projects based on the capacity, resiliency, and reliability of the District's current distribution system and supply sources to ensure that the District can continue to reliably and cost-effectively serve its customers through 2045. The scope of work for this update included a simplified master planning process intended to streamline the refinement of the CIP.

1.1 Project Scope

The scope of the WMP and CIP Update included the following:

- An updated summary and description of the District's water service area and existing water system, including new as-built records for projects completed since 2019;
- An assessment of existing and projected water demands by pressure zone which has been updated using the Global Water Supply Assessment Tracking Tool and the 2020 Urban Water Management Plan (EKI, 2021a);
- An assessment of the District's existing water supply, storage, and pumping capacities and condition, incorporating updated storage criteria used for the *Evaluation of Storage and Supply Requirements for Glen Ellen* (EKI, 2021b);
- Updates to the hydraulic model to assess the existing water system's ability to deliver existing and future water demands and fire flows and identify potential capital improvements to improve system operation;
- Development, prioritization, cost estimation of recommended capital improvements; and,
- Preparation of the Draft and Final Water Master Plan and Capital Improvement Plan Update Report.

1.2 Previous Evaluations and Planning Studies

The current CIP Update references the following planning studies, including:

- 2019 Water Master Plan (EKI, 2019);
- 2020 Urban Water Management Plan (EKI, 2021a);
- 2021 Evaluation of Storage and Supply Requirements for Glen Ellen (EKI, 2021b);



1.3 Report Organization

The CIP Update report is organized following the same format as the 2019 WMP, with sections streamlined to highlight key updates, including:

- Executive Summary
- Section 1 Introduction
- Section 2 Background
- Section 3 Existing Water System Facilities
- Section 4 Existing and Future Water Demands and Fire Flow Requirements
- Section 5 Water Supply and Storage Capacity Evaluation
- Section 6 Water Distribution System Performance and Sizing Criteria
- Section 7 Water Distribution System Modeling Evaluation
- Section 8 Recommended Capital Improvement Program
- Section 9 References



2 BACKGROUND

This section describes the physical characteristics of the District's water service area, as well as the current and projected population for the service area. Since the preparation of the 2019 WMP the District's service area and demographics have not changed significantly.

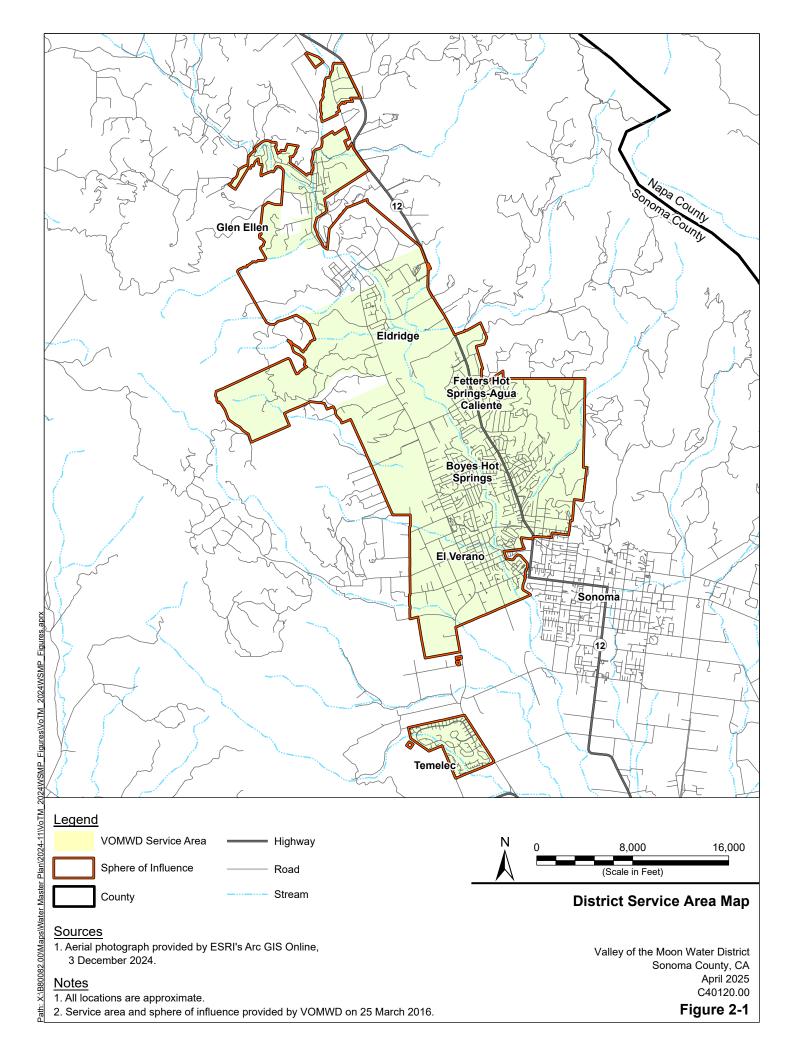
2.1 Site Location and History

The District's service area is located in Sonoma County, approximately 50 miles north of San Francisco, and is adjacent to the City of Sonoma. As shown in Figure 2-1, the District's water service area extends from the Trinity Oaks Subdivision in the north to the Temelec Subdivision in the south. The service area encompasses approximately 11.8 square miles and includes residential and commercial customers. Elevations in the service area range from approximately 60 feet above mean sea level to approximately 1,190 feet above mean sea level.

The District's Sphere of Influence, a boundary determined by the Sonoma Local Agency Formation Commission (LAFCO) indicating the likely eventual limits of the District's service area, was amended in October 2017 to include areas beyond the District's current service area. As shown in Figure 2-1, the District's Sphere of Influence now also includes the following areas outside of the water service area:

- 1. Territory served by the Sobre Vista Mutual Water Company (SVMWC); and
- 2. Territory occupied by the Sonoma Developmental Center (SDC), which currently owns and operates a municipal water supply, treatment, and distribution system on the campus.





2.2 Service Area Climate

The District's service area has a climate that is typical of the Napa County and Sonoma County areas, characterized by summers that are dry and warm and winters that are relatively mild with most rainfall occurring during this season. The regional averages for reference evapotranspiration (ETo), rainfall, and temperature are summarized in Table 2-1.

Month	Reference Evapotranspiration, ETo (a)	Average Rainfall (b) (inches)	Average Temperature (b)			
	(inches)		Min (°F)	Max (°F)		
January	1.0	6.14	37.2	57.2		
February	1.6	5.27	39.9	63.2		
March	3.0	4.05	40.8	66.4		
April	4.5	1.77	42.3	71.2		
May	5.6	0.82	46.0	77.2		
June	6.6	0.23	49.7	84.1		
July	7.1	0.03	51.2	88.6		
August	6.3	0.08	50.8	88.2		
September	4.7	0.33	49.3	86.3		
October	3.3	1.67	45.5	78.6		
November	1.5	3.85	40.6	65.9		
December	1.0	5.18	37.1	57.5		
Annual	46.1	29.4	44.2	73.7		

Table 2-1	. Climate	Characteristics
-----------	-----------	-----------------

Notes:

(a) Reference evapotranspiration data for Valley of the Moon from Appendix A of the California Code of Regulations, Title 23, Division 2, Chapter 2.7, Model Water Efficient Landscape Ordinance, 15 July 2015.

(b) Precipitation and temperature data for the Sonoma Climate Station (048351) from the Western Regional Climate Center for the period 1 January 1893 through 31 May 2016.

2.3 Number of Service Connections

Table 2-2 and Figure 2-2 summarize the number of customer service connections in each water use category between Fiscal Year (FY) 2016 and FY 2024. Customers in the District's service area are classified by the following categories:

- Single-Family Residential (SFR);
- Multi-Family Residential (MFR);
- Commercial;
- Institutional; and,
- Irrigation (MFR and Commercial).

The SFR category comprises approximately 90% of all customer service connections in the District. The number of SFR service connections has slightly grown by 19 (0.3%) since FY 2017. The number of Commercial and Institutional and Irrigation connections has remained stable over the past eight years.





Table 2-2

Number of Current and Historical Potable Water Services by Customer Category

Water Use Sector	Number of Potable Water Service Connection (a)									
	FY 2016	FY 2017	FY 2018	FY 2019	FY 2020	FY 2021	FY 2022	FY 2023	FY 2024	
Single Family Residential	6,226	6,239	6,235	6,220	6,230	6,235	6,247	6,256	6,258	
Multi-Family Residential	439	440	446	446	448	446	448	448	447	
Commercial	166	172	171	172	170	171	165	163	172	
Institutional / Governmental	33	34	33	33	33	34	34	34	35	
Irrigation Multi-Family	20	22	22	22	24	24	24	24	24	
Irrigation Commercial	12	13	12	11	11	12	12	13	14	
Other / Construction	21	24	24	23	21	21	17	18	21	
Total Number of Services	6,917	6,944	6,943	6,927	6,937	6,943	6,947	6,956	6,971	

Abbreviation:

FY - fiscal year

Notes:

(a) Number of service connections for FY 2016 - 2017 and FY 2018 - 2024 were processed from water billing data provided by the District on 1 August 2018 and 30 October 2024, respectively, based on the number of unique billing data accounts in each land use classification.

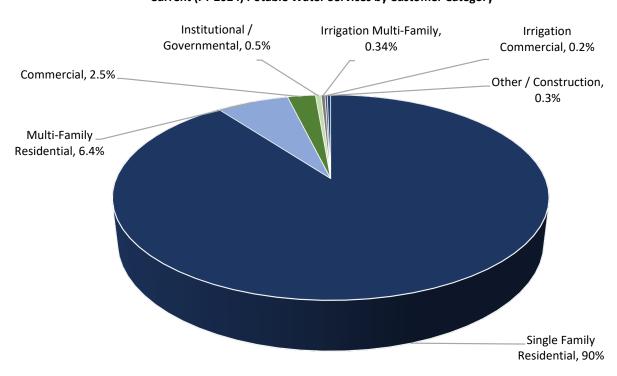


Figure 2-2 Current (FY 2024) Potable Water Services by Customer Category

2.4 Service Area Demographics

The demographics of the District's customers include a wide range of income, household size, and water demands. Typically, the more affluent households are located along the foothills and are characterized by larger lots and homes with higher water demands for irrigation. On the other end of the spectrum, there are two disadvantaged communities located within the District which tend to have smaller lots and lower water use.

Due to the District's setting in the heart of a tourist destination, Sonoma Valley, another factor impacting water use in recent years has been the increase in the number of second homes and vacation rentals. These accounts tend to have higher water use because the sites do not have fulltime owners looking for leaks and managing irrigation water use in accordance with weather patterns.

2.5 Current and Projected Population

The current 2024 population was estimated using a persons-per-connection method consistent with methodology used in the District's 2020 Urban Water Management Plan (UWMP). A population estimate was obtained by compiling population estimates from the persons-per-connection method. The persons-per-connection factor for the District per the State Water Board Division of Drinking Water (DDW) Electronic Annual Report (EAR) was 3.3. Using this methodology, the District's existing service area population is estimated to be 23,004.

The 2020 UWMP assumed a 1.5% population growth per year compared to the 2020 population. The current 2024 population is on track with the estimated 2025 population. The existing and projected service area population based on the UWMP projections are summarized in Table 2-3.

	2024 (Existing)	2025	2030	2035	2040	2045
Population Estimates (a)	23,004	24,860	26,782	28,851	31,081	33,483

Table 2-3. Population Projections

Notes:

 (a) The 2024 population is calculated based upon a persons-perconnection method assuming a persons-per-connection factor of 3.3 per the District's 2020 Urban Water Management Plan (2020 UWMP). Projected populations are based on the District's 2020 UWMP.

2.6 Anticipated Future Development

The District anticipates providing connections and service to the following major developments in the future:

- <u>810 West Agua Caliente Road</u>: the proposed development includes the construction of a hotel, townhomes, affordable housing units, and a retirement community on vacant land at the intersection of Arnold Drive and Agua Caliente Road (EKI, 2021a);
- <u>Verano Ave Multi-Family Residential Development</u>: An 80-unit multi-family development on Verano Avenue across from Maxwell Farms Regional Park anticipated to be completed by 2025; and,



• <u>The Springs Specific Plan</u>: the proposed development is bounded by Agua Caliente Road at the north and Verano Avenue at the south and bisected by the Highway 12 commercial corridor includes up to an additional 124 single-family dwellings units, 561 multi-family or live-work dwelling units, 167,000 square feet of commercial space, 120 hotel rooms, 82,000 square feet of office space, and 27,000 square feet of recreational area anticipated to be completed over the next 50 years.

Water demand projections associated with these developments are discussed further in Section 4.

Additional development is anticipated within the District's sphere of influence as part of redevelopment of the SDC Site. The SDC Site is currently owned by the State of California. The SDC Site has rights and access to local surface water supplies and associated treatment, storage, and distribution infrastructure. The SDC water system was previously permitted and operated as a public water system but was shut down in 2019 when the SDC was closed. The SDC Site is located within the District's sphere of influence, and the District plans to annex the SDC area and incorporate the SDC water system into its system when the site is redeveloped. EKI recently supported the District's planning efforts related to serving the SDC Project. In 2022 and 2023, EKI prepared the following planning documents in support of the SDC Project:

- 1) The Water Supply Assessment for the SDC Specific Plan (EKI, 2022);
- 2) A hydraulic assessment that evaluated how the SDC water supply infrastructure could be integrated with the District's water system (EKI, 2023a);
- 3) A conceptual transition plan that outlined steps to transition control of SDC water rights, facilities, and operations to the District (EKI, 2023b); and,
- 4) A peer review of the Sonoma Developmental Center Water System Assessment Report prepared by Wood Rodgers and dated April 2023.

The development will likely include between 200 and 500 dwelling units. However, the timeline for this future development is uncertain and it is not currently with the District's service area. Thus, the SDC is not included in this master planning effort.



3 EXISTING WATER SYSTEM FACILITIES

The District's water supply is provided by the Sonoma County Water Agency (SCWA) and groundwater wells owned and operated by the District. The District owns, operates, and maintains the potable water distribution system that serves drinking water to its residential, commercial, and institutional customers. This section summarizes the District's water supply facilities and distribution system, including water storage, pumping facilities, and pipe network. A map of the District's existing water system is shown on Figure 3-1 and a hydraulic profile schematic of the District's water system is shown on Figure 3-2.

The District's water system facilities have been updated based on completed and in-progress CIP projects, many of which were identified in the 2019 WMP, including:

- The addition of the Pedroncelli and Craig groundwater wells
- The installation of the new Saddle Tank
- Modifications to the District's pressure reducing valve (PRV) settings
- Installation of 7,715 linear feet of pipe based on approximately 37 individual as-builts listed as project groups below:
 - EST 2947 The Walnut Ave, Oak St. & Penny Ln. Water Main Replacement Project
 - <u>EST 2958</u> Aqua Caliente Road, Arnold Drive, and Summer Meadow Lane Main Replacement Project (including a new normally closed cross connection between PZ1 and PZ1B)
 - EST 2967 The Boyes Blvd. Bridge Pipeline Replacement Project
 - EST 2983 The Pedroncelli and Craig Well installations
 - <u>EST 2984</u> The Gibson St., Riddle Rd Easement, Sobre Vista (near Lake Josephine), Brookview & Lomita Steel Water Main Replacements
 - <u>EST 2993</u> The Fetters Ave., Malek Rd., Depot Rd., Sobre Vista Dr., and Wake Robin Rd. Main and Service Line Replacement Project
 - <u>EST 2996</u> the Arnold Dr Main Replacement Project
 - EST 3021 the Waterline Improvement Project on Chestnut Rd to Chestnut Tank
 - EST 3034 to EST 3040 Various service line, and hydrant installations
- Upcoming capital improvement projects currently in design or construction, including:
 - Altimira Fire Flow Improvement Project
 - Verano Hotel Private Water Improvements;
 - 18661 Lomita Avenue New Water Main Project
 - Boyes Food Center Mixed Use Development Water Main Improvement Project

3.1 Water Supply Facilities

The District, along with seven other cities and special districts in Sonoma and Marin County, has a water supply agreement with Sonoma County Water Agency (SCWA) for the purchase of Russian River water. The majority (approximately 79% under normal water year conditions) of the District's water supply comes from SCWA Russian River water purchases. The District's remaining water is supplied by eight (8)



groundwater wells that are owned (or leased) and operated by the District. The District's imported water and groundwater supply facilities are described in detail below.

3.1.1 SCWA Supply and Transmission System

The District's water supply contract with SCWA, known as *Restructured Agreement for Water Supply* (Restructured Agreement), was executed in 2006 and entitles the District to 8.5 million gallons per day (MGD) during any month and an annual maximum of 3,200 AFY. Provided the supply is available, the Restructured Agreement permits the District to take delivery of water more than its entitlement during a given month, provided specific conditions specified in the Restructured Agreement are met.

The SCWA's storage and transmission system is shown on Figure 3-3. As described below, the system includes lakes, streams, rivers, aqueducts, tanks and other facilities.

The SCWA storage and transmission system is supplied water from the natural flow of the Russian River. The Russian River water is stored in Lake Sonoma, behind Warm Springs Dam, and in Lake Mendocino, behind Coyote Dam. These dams are federal projects under the jurisdiction of the U.S. Army Corps of Engineers (USACE). SCWA is the local sponsor and partners with the USACE for the water supply portion of the reservoir projects. SCWA owns and operates the water supply pools at both Lake Sonoma and Lake Mendocino. The design water supply pool capacities of Lake Sonoma and Lake Mendocino are 245,000 AFY and 122,500 AFY, respectively.

The SCWA uses approximately 14 miles of the natural channel of Dry Creek and approximately eight miles of the Russian River to convey water from Lake Sonoma to its diversion facilities. Water is diverted and extracted from the stretch of river located just upstream of Wohler Bridge and downstream of Mirabel via six Ranney Collectors. The diverted river water percolates through sand and gravel and only needs the addition of chlorine to meet the California Drinking Water Program drinking water quality standards. A system of aqueducts, booster pumps, and tanks then distribute the water to the various Water Contractors and other transmission system users. The transmission system was designed to meet peak day demands of its customers.

SCWA also owns and operates three groundwater supply wells located in the Santa Rosa Plain Subbasin of the Santa Rosa Valley Groundwater Basin. These groundwater wells are located along the Russian River-Cotati Intertie Pipeline and are used to supplement the SCWA water supply.

The SCWA Restructured Agreement established a goal for each water contractors including the District to supply and maintain approximately 40% of its maximum month demand through local sources to mitigate against drought, emergencies, and temporary Transmission System outages.

The District's SCWA supply is conveyed from the Sonoma Aqueduct, which is owned and operated by the SCWA and serves both the District and the City of Sonoma. Storage in this portion of the Sonoma Aqueduct is provided by the Annadel Tanks located upstream of the District near Oakmont, the Eldridge Tanks located between the northern and southern portions of the District south of Glen Ellen, and the Sonoma Tanks located downstream of the District and serve the City of Sonoma. Pressure for the aqueduct in this region is provided by Sonoma Booster Pump Station No. 1 and Sonoma Booster Pump Station No. 2, located on the east side of Spring Lake. The Eldridge Booster Pump Station located at the Eldridge Tank is typically off-line.

The District is supplied through 10 turnouts that are spread along the aqueduct from just north of Trinity Road and Highway 12 south to Verano Avenue and Fifth Street West near the City of Sonoma (see Figure 3-1). Two turnouts are located upstream of the SCWA Eldridge Tanks (Glen Ellen and Trinity Oaks turnouts), and the remaining eight turnouts are located downstream of the Eldridge Tanks. The SCWA meters water to the District at each turnout. Pressure available at each turnout depends on the hydraulic



conditions along the aqueduct and the fill cycles of the SCWA Eldridge and Sonoma Tanks. Pressures are typically substantially lower at the turnouts when the tanks are filling. Detailed information regarding each turnout is listed in Table 3-1.

Downstream of each SCWA turnout, the District owns and operates pressure reducing valve (PRV) stations. These stations are described in Section 3.4.2.

3.2 Groundwater Supply

The District supplements its purchased SCWA water with the use of local groundwater. The District owns and/or operates a total of eight active municipal production wells.¹ Portions of the District are located within the Sonoma Valley Subbasin, which is identified by the California Department of Water Resources (DWR) as 2-002.02 and is a subbasin of the Napa-Sonoma Valley Groundwater Basin (DWR 2-002). As shown in Figure 3-4, four of the District's existing wells (Verano, Larbre, Craig, and Pedroncelli) are located within Sonoma Valley Subbasin; the other wells are outside of the DWR-defined groundwater basins.

3.2.1 Groundwater Management

The Sonoma Valley Subbasin is not adjudicated and has not been identified by DWR as a criticallyoverdrafted groundwater basin. The Basin is listed as a high priority and is subject to mandatory management under the Sustainable Groundwater Management Act of 2014 (SGMA) requirements. The Sonoma Valley Groundwater Sustainability Agency (GSA) was formed in June 2017 through a Joint Exercise of Powers Agreement (JPA) between the District and the City of Sonoma, North Bay Water District, Sonoma Resource Conservation District, SCWA, and County of Sonoma.

The Groundwater Sustainability Plan for the Sonoma Valley Groundwater Subbasin (GSP) was submitted to the California Department of Water Resources in January 2022 and approved by DWR on January 26, 2023.² The GSP establishes a standard for sustainability of groundwater management and use and determines how the basin will achieve this standard by 2042.

3.2.2 Groundwater Wells and Treatment

The District's active groundwater wells are summarized in Table 3-2. The capacities of the District's wells range from 90 gallons per minute (gpm) to 300 gpm. The District cycles through its wells; each well typically pumps for nine months and then remains offline for a three-month recovery period.

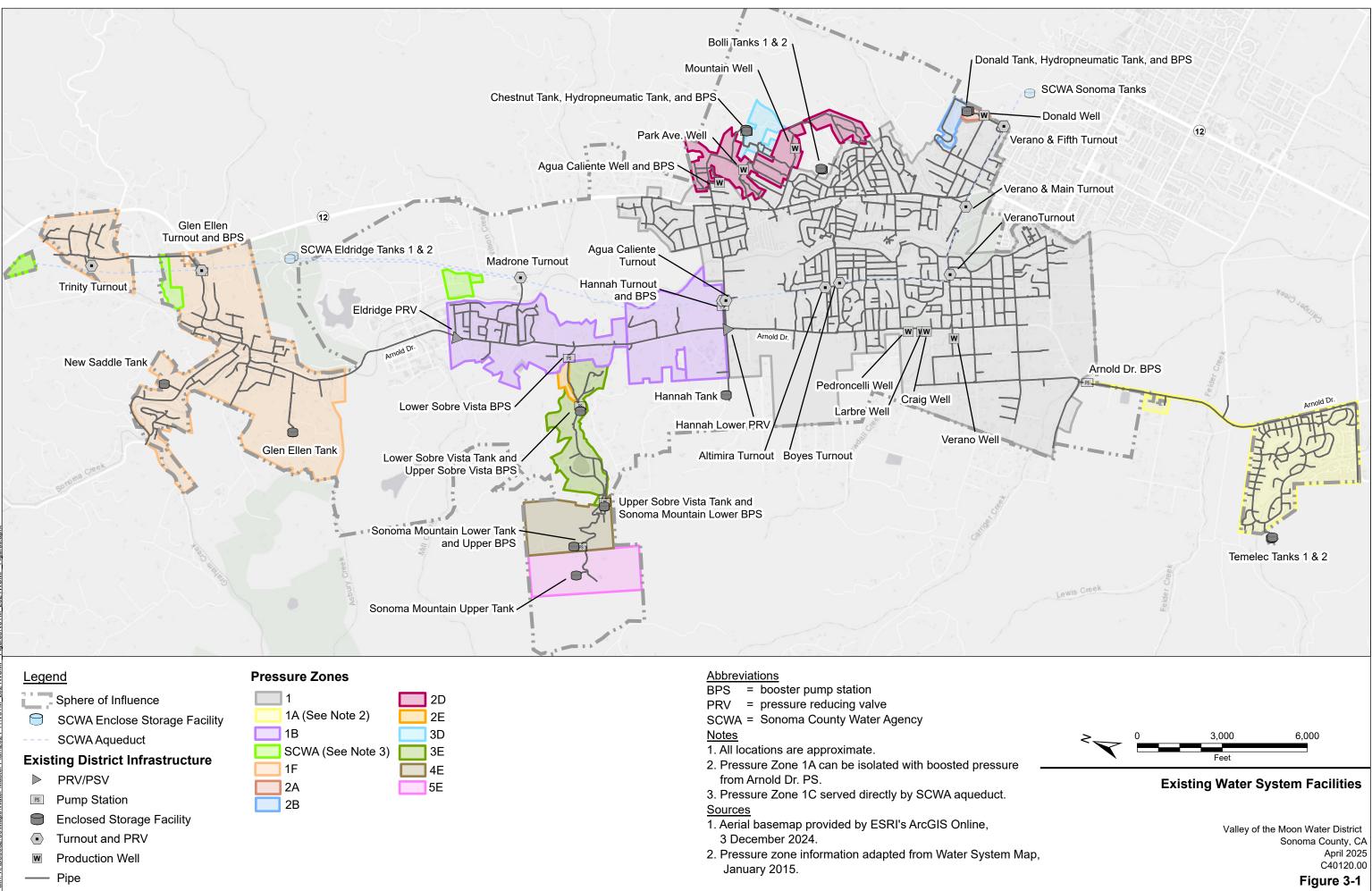
The District's "Well 5A" or "Verano Well" is currently not used due to water quality issues that cannot be treated with the existing iron and manganese treatment system.³ However, the District is currently implementing an Aquifer Recovery and Storage (ASR) project at the Verano Well and Park Avenue well sites, which may allow for reactivation of the Verano Well. The ASR Project is currently in the pilot testing phase and construction of permanent ASR facilities at the two well sites (if pilot testing is successful) is anticipated to be completed by early 2026. The ASR project is expected to provide enhanced water supply reliability during droughts, natural disaster events, and seasonal periods of peak water demand.

https://sonomavalleygroundwater.org/gsp/

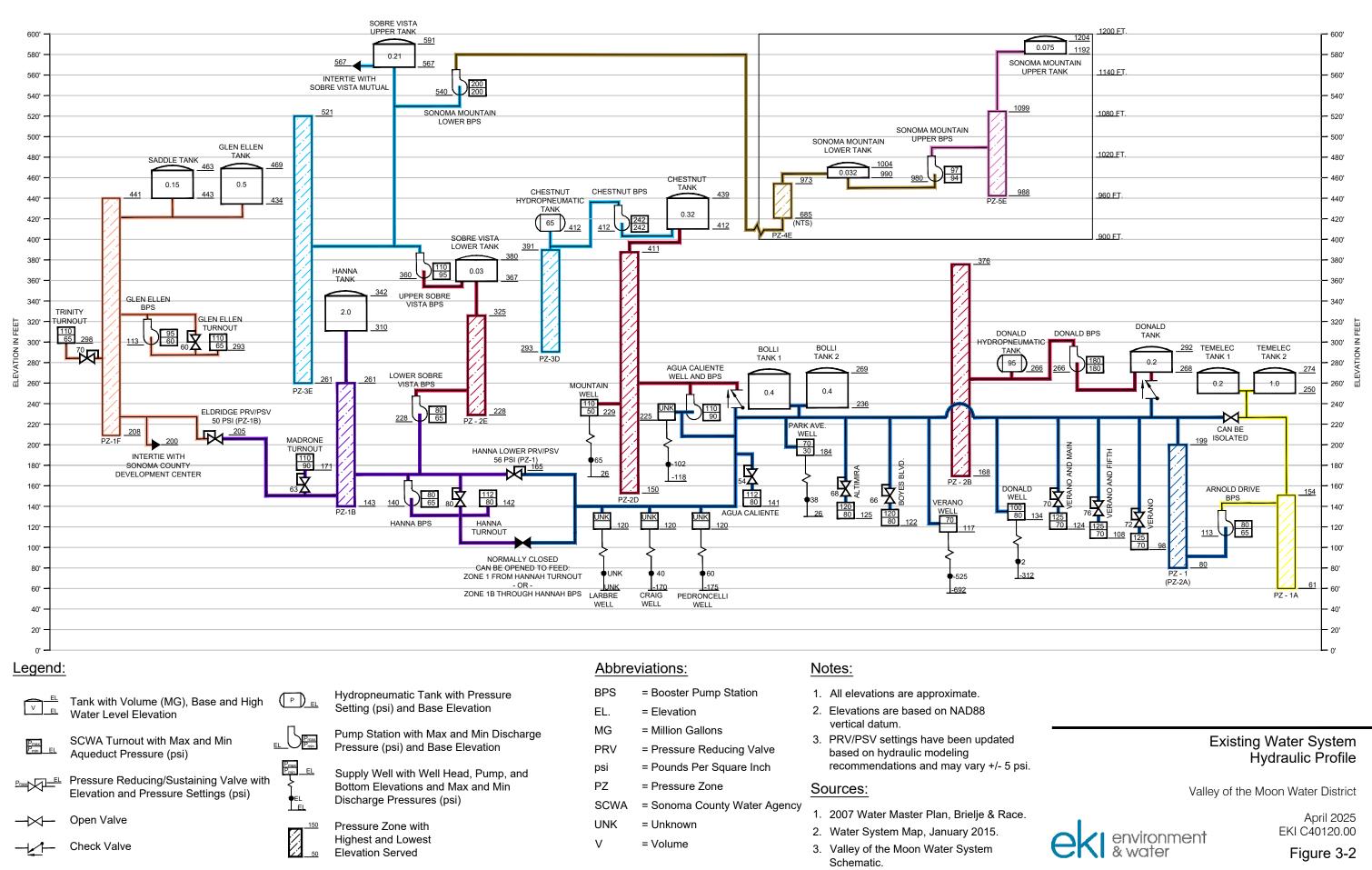
¹ The District also owns a ninth well, Trinity Oaks Well, which is inactive. There are no plans to reactivate this well.

² The GSP and the letter of determination from DWR can be viewed here:

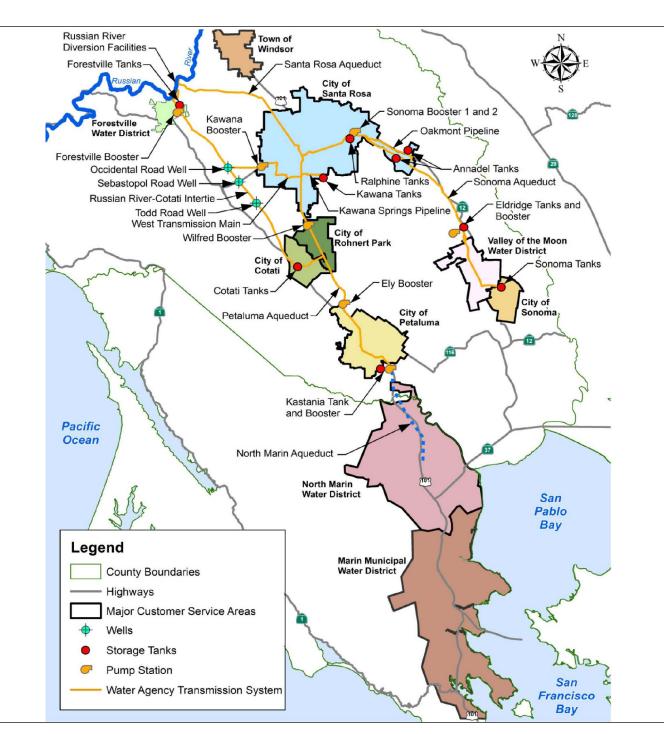
³ Water quality issues include elevated arsenic and temperature, and hydrogen sulfide odors.



2.	Pressure zone i	nformation	adapted	from V	Vater S	system I
	January 2015.					



	······································
3.	Valley of the Moon Water System
	Schematic.



Notes

- 1. All locations are approximate.
- 2. Not to scale.

Sources

1. Sonoma County Water Agency 2010 Urban Water Management Plan

SCWA Service Area and Water Transmission System Facilities

Valley of the Moon Water District Sonoma County, CA April 2025 C40120.00 & water Figure 3-3



Table 3-1 SCWA Turnout Information

SCWA Turnout	Pressure Zone Served	Meter Size (in)	Turnout Elevation (ft)	SCWA A Pressure (ps	Associated District PRV Station (b)	
	Scived		(10)	Low	High	Station (S)
Verano	1	6	98	70	125	PRV-1
Verano and Main	1	10	124	70	125	PRV-2
Verano and Fifth	1	6	108	70	125	PRV-3
Boyes Boulevard	1	6	122	80	120	PRV-4
Altimira	1	6	125	80	120	PRV-5
Agua Caliente	1	6	141	80	112	PRV-6
Hanna	1B	10	142	80	112	PRV-7
Madrone	1B	6	171	90	110	PRV-9
Glen Ellen	1F	6&4	293	65	110	PRV-11
Trinity Oaks	1F	6	298	65	110	PRV-12

Notes:

- (a) Pressure in aqueduct at each turnout location vary based on the SCWA tank filling cycles. Pressure ranges are approximate.
- (b) The District operates PRV stations directly downstream of each turnout location to reduce pressures as necessary from the aqueduct pressures. Refer to Table 3-5 for PRV station information.

Lastly, in 2023, the District drilled an 800 feet deep exploratory borehole adjacent to its Chestnut Tank to determine the feasibility of installing a permanent production well at this location. Zone testing was performed at several intervals to determine potential yield and water quality of a future well and it was determined that a well at the site would likely be capable of producing over 100 gpm, however several potential water quality concerns were noted (EKI, 2024b).⁴ The District is not currently pursuing a well at this location, however, may do so in the future.

3.3 Emergency Interties

The District has emergency interties with the SDC and Sobre Vista Mutual Water Company (SVMWC). In the event of an emergency, either the District can supply SVMWC or SDC with potable water under the terms of their agreements. The District can also receive water from SDC, although the SDC is currently being served via the SCWA Aqueduct instead of its local surface water supplies and thus would not be able to supply the District should the SCWA Aqueduct be impaired.

3.4 Water Distribution System Pressure Zones and Facilities

The District's existing water distribution system facilities include pipe network, PRV stations, water storage facilities, booster pump stations, and other features. The District's distribution system consists of several pressure zones. The District's distribution system facilities are discussed in the following sections.

3.4.1 Pressure Zones

The District's water distribution system has 11 pressure zones as shown on Figure 3-1. The District's service area also includes several customers that are supplied with water directly from the SCWA aqueduct. The majority of the District's customers that are located on the valley floor are served from the SCWA aqueduct pressure (Pressure Zones 1, 1A, 1B, 1F) while customers in the higher elevations of the Sonoma Valley are served by separate pressure zones. Pressures Zones 1 and 1A are typically operated as a single pressure zone, but the District can isolate Pressure Zone 1A by pumping from Zone 1 via Arnold Drive Pump Station if there is insufficient aqueduct pressure. Under certain conditions, booster pump stations are needed to supply flow to Pressure Zones 1B and 1F, as discussed in Section 3.4.3. Minimum and maximum service point elevations are shown on Figure 3-2 and summarized in Table 3-3.

The number of services in each zone is listed in Table 3-3 and Table 3-4. Approximately 68% of service connections are in Pressure Zone 1 and 94% of connections in total are in the lower aqueduct zones (Pressure Zones 1, 1A, 1B, and 1F). All of the District's commercial, institutional, and landscape accounts are located in the lower aqueduct zones (Pressure Zones 1, 1A, 1B, and 1F).

3.4.2 Pressure Reducing Valve Stations

The District maintains 13 PRV Stations in its system. Ten of the PRV stations are installed directly downstream of each SCWA turnout to control the pressures delivered from the aqueduct to the District's distribution system. In addition, the district operates the Eldridge PRV to separate Pressure Zones 1F and 1B and the Hanna Lower PRV to separate Pressure Zone 1B and Pressure Zone 1. These valves typically remain close but are set to open in case of an emergency to deliver flow from Pressure Zones 1F to 1B through the Eldridge PRV and from Pressure Zone 1B and 1 through the Hanna PRV. Information on the District's PRV Information is provided in Table 3-5.

⁴ Water quality concerns include elevated manganese and water temperature.



3.4.3 Water Storage Facilities

As shown in Table 3-6, the District has approximately 5.5 million gallons (MG) of total storage in 13 groundlevel storage tanks. The District's tanks are used to help meet system demands during peak hours, provide emergency storage, and provide fire flow storage. During the October 2017 Sonoma County wildfires, the District's Saddle Tank was destroyed but has since been reconstructed in 2020 with a new 0.15 MG welded steel tank.

3.4.4 Booster Pump Stations and Hydropneumatic Tanks

As shown in Table 3-7, the District has 10 booster pump stations (BPSs) to serve upper pressure zones and fill upper storage tanks. The Hanna and Glen Ellen BPSs operate to boost pressure from the SCWA aqueduct during certain operating conditions to supply Pressure Zones 1B and 1F, respectively. Seven BPSs have backup power on-site and the District has portable generators that it uses to power the remaining BPSs during power outages. Hanna Pump Station is the only BPS with variable frequency drive pumps; all other pumps are constant speed.

Hydropneumatic tanks are installed at Donald and Chestnut BPSs to supply Pressure Zones 2B and 3D, respectively, during lower demand periods to limit cycling the BPSs. Characteristics of these hydropneumatic tanks are described in Table 3-7.

3.4.5 Water Mains and Other Distribution System Features

The District's water distribution network consists of approximately 92 miles of pipe ranging from <2 inches to 14 inches in diameter. Pipe materials are primarily asbestos cement pipe (ACP) and polyvinyl chloride (PVC) pipe, but there are also sections of cast-iron pipe (CIP), ductile iron pipe (DIP), steel pipe, and high-density polyethylene (HDPE) pipe. Table 3-8 summarizes the District's distribution pipelines by diameter and material. The District's standard material for new pipe installations is PVC pipe. Distribution system pipe sizes are shown on Figure 3-5 and materials are shown on Figure 3-6.

The Districts distribution system also contains valves, blow offs, air release valves, hydrants, service connections, meters, and other appurtenances necessary to reliably operate the system.

Since publishing the 2019 WMP, the District has replaced approximately 2,820 feet of steel pipe. The majority of the remaining steel pipes are small diameter service connections, but a few steel bridge crossings remain.

3.4.6 SCADA System

The District has a supervisory control and data acquisition (SCADA) system monitoring all wells, pump stations, storage tanks, hydropneumatic tanks. The system allows the District to remotely control operational setpoints and monitors for discharge flows, suction and discharge pressures, tank levels, pump runtimes, and chlorine residual. Alarms are set for minimum and maximum levels and pressures. The District also monitors levels in and flows from the SCWA Eldridge Tank and flows in the Aqueduct upstream and downstream of the District.

Between 2016 and 2017, the District upgraded the SCADA System and added tank hatch alarms and video cameras.

Since the 2019 WMP, the District has not made any major changes the SCADA system but has incorporated the new well and tank facilities into the SCADA system.



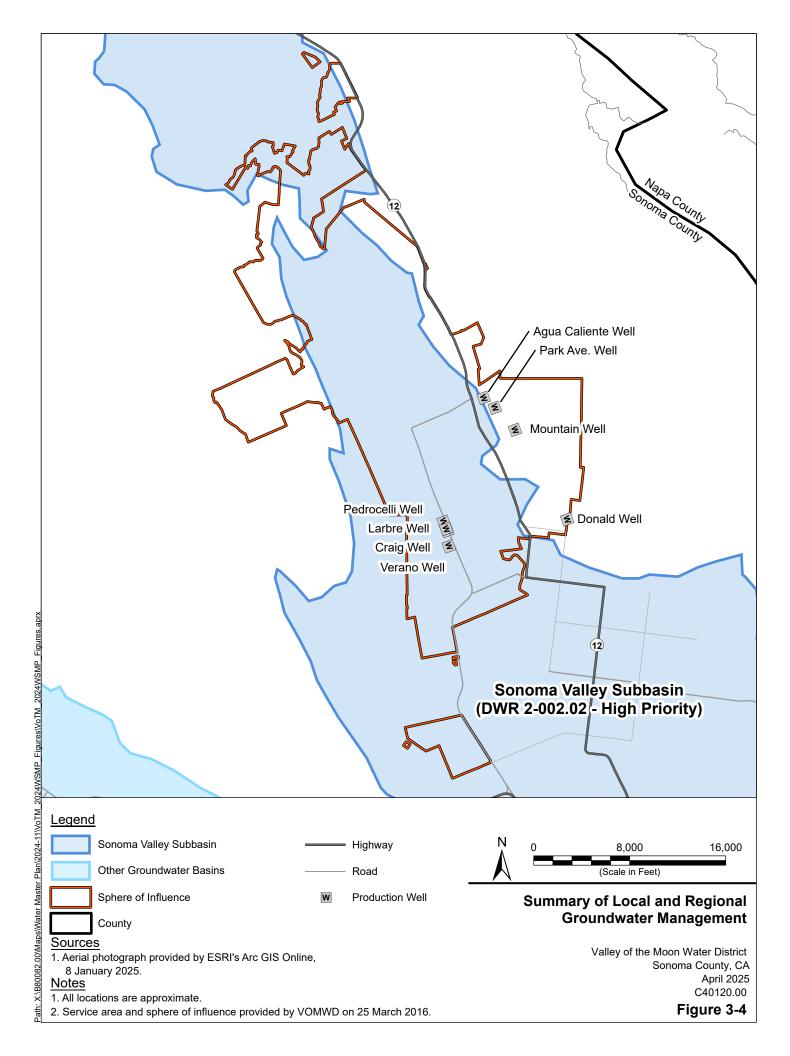




Table 3-2 Groundwater Well Information

Well Name	Pressure Zone Served	Ground Surface Elevation (ft)	Pump Elevation	Bottom of Casing Elevation (ft)	Design Flow Rate (gpm)	Motor Horsepower	Design Head (ft)	Backup Power	Treatment
Existing Wells									
Donald	1	134	2	-312	110	15	288	No	Chlorination - Chlorine Tablets
Mountain Ave.	2D	229	65	26	110	15	290	Yes	Chlorination - Chlorine Tablets
Park Ave.	1	184	38	26	90	7.5	370	No	Chlorination - Chlorine Tablets
Agua Caliente	1	225	-102	-118	120	25	370	Yes	Chlorination - Chlorine Tablets
Verano	1	117	-525	-692	300	50	400	No	Iron/Manganese/Arsenic Treatment - Green Sand Filtration
Larbre	1	120			110	15	200	No	Grit Removal Chlorination - Chlorine Tablets
Pedroncelli (a)	1	120	60	-175	150			No	Chlorination - Chlorine Tablets
Craig (a)	1	120	40	-170	125	15	375	No	Chlorination - Chlorine Tablets
Future Wells		•						•	•
Park Ave. (New)(b)	1	184	15	-116	125				Chlorination - Chlorine Tablets

Notes:

(a) Information shown for the Pedroncelli and Craig wells per Permit Application by EKI dated May 2022. Due to flow from the upper to lower aquifer, the Pedroncelli well was grouted to a depth of 295 ft and the Craig well was grouted to a depth of 290 ft. Additionally, per discussions with the District the expected flowrate for the Pedroncelli well has been reduced to 150-gpm due to air entrainment.

(b) The new Park Well is currently being installed and is expected to be in service by mid-2025. Design information shown based on the Aquifer Storage and Recovery Drilling Phase design plans by EKI dated Jan 2023.

Abbreviations:

gpm = gallons per minute

ft = feet



Table 3-3 Pressure Zone Information

Pressure Zone	Service Point E	Service Point Elevation (ft) (a)						
Pressure zone	Min	Мах	Connections (FY2024)					
1	80	199	4,767					
1A (b)	61	154	806					
1B	142	243	422					
1F	208	441	538					
2A (b)	134	212	2					
2B	168	376	27					
2D	150	411	329					
2E	228	325	2					
3D	293	391	27					
3E	261	521	34					
4E	685	973	0					
5E	988	1,099	3					

Notes:

(a) Elevations are approximate and based on NAD88. Service points include service connections or hydrants.

(b) Pressure Zones 1A and 2A typically operate as part of Pressure Zone 1 but can be isolated, if necessary.



Table 3-4

Current and Historical Potable Water Services by Pressure Zone and Customer Classification

	Number of Potable Water Service Connections by Pressure Zone												
Sector	1	1A	1B	1F	2A	2 B	2D	2 E	3D	3E	5E	Hydrant Meters	SCWA (a)
Single Family Residential	4,200	793	393	465	2	27	318	2	25	26	3		4
Multi-Family Residential	369	2	17	38			11		2	8			
Commercial	126	7	5	25									
Institutional	27		3	4									1
Irr. Multi-Family Residential	18	4	1	2									
Irr. Commercial	11		1	1									
Other/Construction	16		2	3								9	
Total Number of Services	4,767	806	422	538	2	27	329	2	27	34	3	9	5

Notes:

(a) SCWA serves these customers directly off of the aqueduct.



Table 3-5
PRV Station Information

PRV ID	Description	Pressure Zone Served	Size (in)	PRV Setting (psi) (a)	Elevation (ft)
SCWA Tu	rnout Pressure Reducing				
PRV-1	Verano Turnout PRV	1	8	76	98
PRV-2	Verano and Main Turnout PRV	1	10	65	124
PRV-3	Verano and Fifth Turnout PRV	1	6	71	108
PRV-4	Boyes Boulevard Turnout PRV	1	6	66	121
PRV-5	Altimira Turnout PRV	1	8	64	125
PRV-6	Agua Caliente Turnout PRV	1	6	57	141
PRV-7	Hanna Turnout PRV	1B	10	80	142
PRV-9	Madrone Turnout PRV	1B	6	63	171
PRV-11	Glen Ellen Turnout PRV	1F	6&4	60	293
PRV-12	Trinity Oaks Turnout PRV	1F	6	70	298
PRV-15	Theodor PRV (b)	(2D)	6	57	190
Other Reg	gulating Valves				
PRV-10	Eldridge PRV/PSV	1F to 1B	6	50 PSI (PZ1B)	213
PRV-13	Hanna Lower PRV/PSV	1B to 1	8	56 PSI (PZ1)	165

(a) Data from 9 September 2024 Pumping and Storage Report provided by Valley of the Moon Water District 30 September 2024. Settings for PZ-1 PRVs updated per District comments provided on 16 January 2025.

(b) The Theodor PRV serves to reduce excessive pressures to approximately ten homes in PZ-2D on Falcon Ln.

Abbreviations:

gpm - gallons per minute in - inch PRV - pressure reducing valve SCWA - Sonoma County Water Agency



Table 3-6 Water Storage Facility Information

Tank Number	Tank Name	Pressure Zones Served (a)	Date Constructed	Material	Capacity (MG)	Height (ft)	Diameter (ft)	Tank Floor Elevation (ft)
T-1	Temelec 1	1A & 1	1985	Welded Steel	1.0	24.0	84	250
T-2	Temelec 2	1A & 1	1966	Welded Steel	0.2	24.0	38	250
T-3	Donald	2B & 1*	1963	Welded Steel	0.2	24.1	40	268
T-4	Glen Ellen	1F, 1B*, & 1*	2006	Welded Steel	0.5	35.0	52	434
T-5	Saddle	1F, 1B*, & 1*	2020	Welded Steel	0.15	24.0	40	443
T-6	Bolli 1	1	2001	Welded Steel	0.4	33.0	46	236
T-7	Bolli 2	1	2001	Welded Steel	0.4	33.0	46	236
T-8	Chestnut	2D, 3D, and 1*	1992	Welded Steel	0.32	27.0	48	412
T-9	Hanna	1B & 1*	1977	Welded Steel	2.0	32.0	104	310
T-10	Sobre Vista - Lower	2E	Pre-1909	Concrete - HDPE Lined	0.03	13.0	20	367
T-11	Sobre Vista - Upper	3E	2002	Bolted Steel	0.2075	24.0	38	567
T-14	Sonoma Mountain - Lower	4E	2006	Bolted Steel	0.032	16.0	18.5	990
T-15	Sonoma Mountain - Upper	5E	2002	Bolted Steel	0.022	8.0	21.6	1192

(a) An asterisk (*) indicates the lower pressure zones can be served indirectly through from this tank through pressure reducing valves.



Table 3-7 Booster Pump Station Information

Booster Pump Station	Pump No.	Pressure Zone (Tank) Served	Head (ft)(a)	Design Flow (gpm)(a)	Horsepower (hp)(a)	Firm Capacity (gpm)	Elevation (ft)	Backup Power	Hydropneumatic Tank Volume (gallons)	Hydropneumatic Pressure Setting (psi)
Arnold Dr.	PS-1	1 to 1A	160	500, 500	20, 20	500	114	No		
Donald (b)	PS-2	1 to 2B	210	100, 100, 350	2, 7.5, 25	450	268	Yes	2,000	95
Chestnut	PS-4	2D to 3D	155	100, 100	5, 5	100	413	Yes	3,000	65
Agua Caliente	PS-5	1 to 2D	205	350, 350	25, 25	350	225	Yes		
Sobre Vista Lower	PS-6	1 to 2E	160	130, 130	7.5 , 7.5	130	229	Yes		
Sobre Vista Upper	PS-7	2E to 3E	105	100, 100	10, 10	100	351	Yes		
Glen Ellen	PS-9	SCWA to 1F	175	450, 450	25, 25	450	290	Yes		
Hanna	PS-10	SCWA to 1B	160	800, 800	20, 20	800	143	Yes		
Sonoma Mntn. Lower	PS-11	3E to 4E	475	26, 26	5, 5	26	535	Yes		
Sonoma Mntn. Upper	PS-12	4E to 5E	220	17, 17	5, 5	17	980	No		

(a) Source: Water Master Plan, 2019.

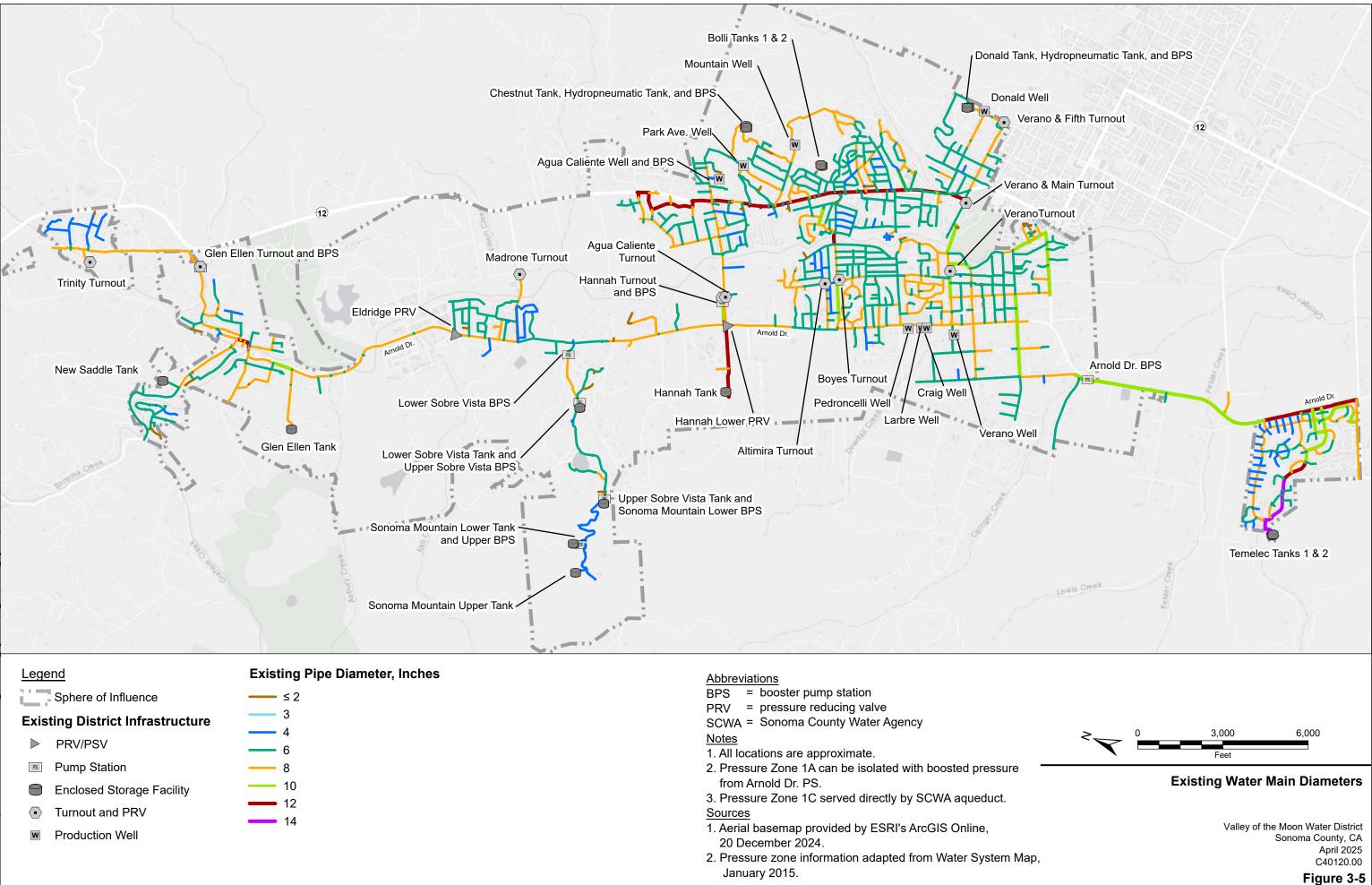
(b) Updated flow and firm capacity of Donald pump station fire pump based on fire flow testing data collected on 18 December 2018.



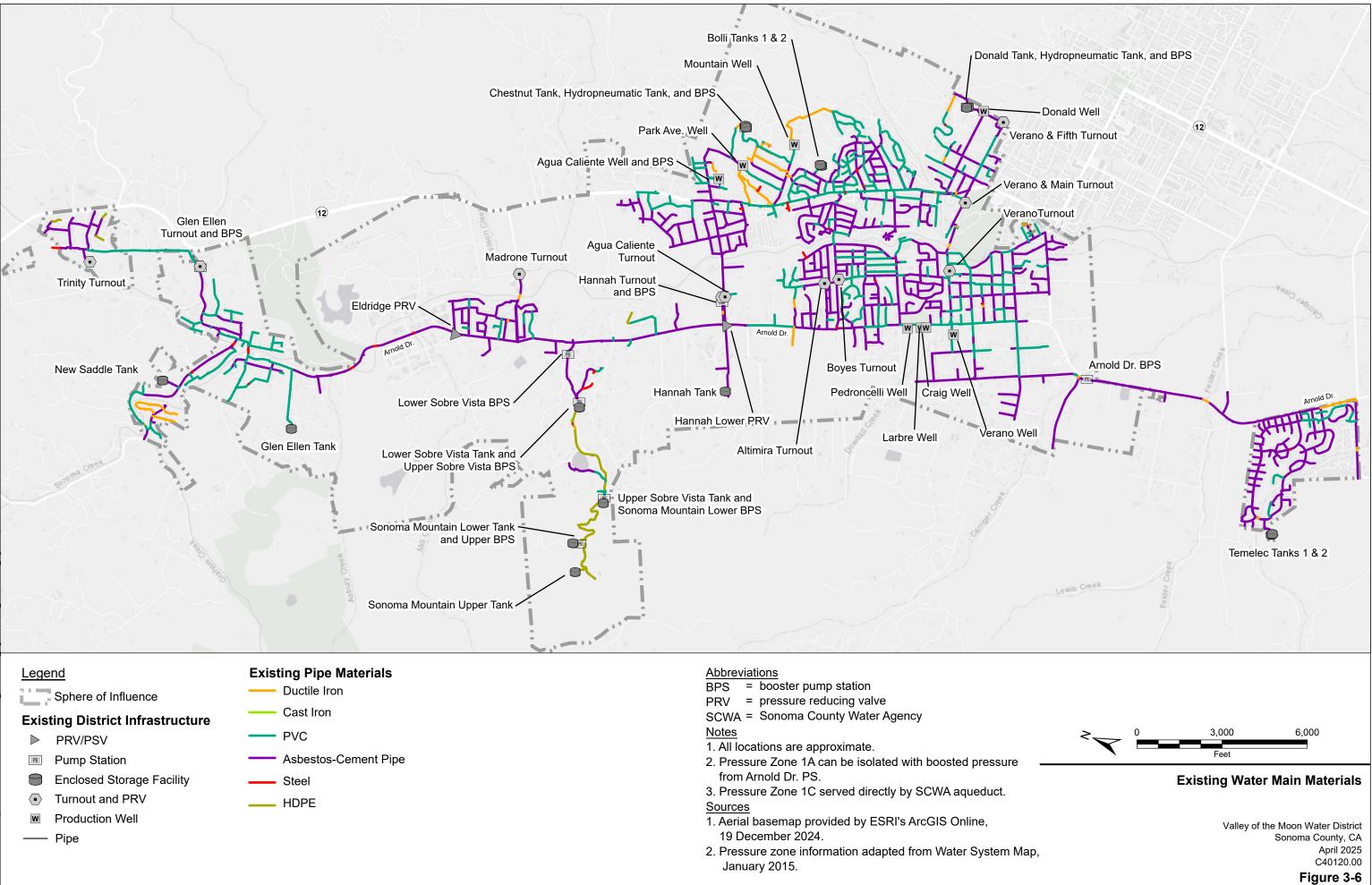
Table 3-8 Pipeline Lengths by Diameter and Material

Pipeline Diameter				Length	Percent of Water System				
(inches)	ACP	CIP	DIP	HDPE	PE	PVC	Steel	(miles)	water system
≤2	94		39	778	424	2,353	1,460	0.97	1.1%
3							20	0.00	0.0%
4	29,655	85	850	6,691		2,602	9	7.56	8.2%
6	159,417		11,177	3,548		58,315	411	44.10	48.0%
8	90,683		4,251			60,248	798	29.54	32.1%
10	19,619		743			6,805		5.15	5.6%
12	7,534		1,464	75		12,660		4.12	4.5%
14	2,692							0.51	0.6%
Total	309,694	85	18,524	11,091	424	142,984	2,698	91.95	100%
Percent of System	63.79%	0.02%	3.82%	2.28%	0.09%	29.45%	0.56%		

(a) Pipeline lengths, diameters, and material includes all active potable water transmission and distribution pipelines present in the AutoCAD files of the Water System Map provided by the District and updated by EKI based on as-built records.



Legend	Existing Pipe Diameter, Inches	Abbreviations
Sphere of Influence	≤2	BPS = booster pump station
Existing District Infrastructure	3	PRV = pressure reducing valve SCWA = Sonoma County Water Agency
▶ PRV/PSV		<u>Notes</u> 1. All locations are approximate.
Pump Station	8 10	 Pressure Zone 1A can be isolated with boosted pressure from Arnold Dr. PS.
 Enclosed Storage Facility Turnout and PRV 	— 12	3. Pressure Zone 1C served directly by SCWA aqueduct.
Iurnout and PRVProduction Well	— 14	<u>Sources</u> 1. Aerial basemap provided by ESRI's ArcGIS Online, 20 December 2024.
		 Pressure zone information adapted from Water System Ma January 2015.



end	Existing Pipe Materials
Sphere of Influence	Ductile Iron
sting District Infrastructure	Cast Iron
PRV/PSV	PVC
Pump Station	Asbestos-Cement Pipe
Enclosed Storage Facility	Steel
Turnout and PRV	HDPE
Production Well	
- Pipe	

4 EXISTING AND FUTURE WATER DEMANDS AND FIRE FLOW REQUIREMENTS

The following sections summarize the District's historical and current water demands, water demand projections, water peaking factors, and fire flow requirements.

4.1 Current and Historical Water Production and Consumption

Table 4-1, Figure 4-1, and Figure 4-2 provide historical context by summarizing the District's potable water use, service area population, and per capita potable water demand for the fiscal years (FY) 1998 through 2024.⁵ Historical water use is based on total annual SCWA water purchases and local groundwater production. Total potable water use in FY 2024 was 2,099 acre-feet (AF). Table 4-2 and Figure 4-3 provide potable water production by supply source between FY 2011 and FY 2024.

Potable water use has generally decreased over the past 20 years, although significant variations have occurred from year to year and are associated with changing hydrologic and economic conditions. The District experienced a decrease in both total and per capita demand in FY 2009 and FY 2010, likely reflecting the impacts of the economic recession. The subsequent increase in water use from FY 2011 to FY 2013 is thought to reflect improved economic conditions. Between FY 2014 and FY 2016, total and per capita water use declined as the drought occurring during this timeframe intensified. The District experienced particularly significant decreases in demand in FY 2015 and 2016, with total potable water demand decreasing by approximately 29% from 2013 demands. This decrease in demand is likely attributable to the severe drought conditions that persisted into 2016 and that resulted in mandatory state-wide restrictions in urban water use imposed by the State Water Resources Control Board (SWRCB). Total and per capita water use increased slightly between FY 2017 through FY 2021, reflecting a partial rebound following the drought. However, once again due to the recent drought, total demand decreased in FY 2022 through FY 2024.

Although population has increased over the past 20 years, per capita water use since 1998 has generally decreased as shown in Figure 4-2. Specifically, per capita water use has dropped to 81 gallons per capita per day (GPCD) in FY 2024, similar to usage in the FY 2016 drought. Per capita demands have been below the District-adopted 2020 Senate Bill x7-7 (SBx7-7) Target of 124 GPCD since FY 2009.

The District-wide current and historical potable water use from FY 2010 to FY 2024 is presented in Table 4-3, Figure 4-4, and Figure 4-5 by individual customer sectors. Table 4-4, Figure 4-6, and Figure 4-7 present current and historical water use by individual pressure zones. Table 4-5 presents demands by pressure zone and customer categories. Water demand within the District's service area is measured using water meters that are installed at each customer account. Records of current and historical water use at each account are maintained by the District and are based on billing data.

Water use in the District's service area is predominantly associated with residential use. Residential customers account for approximately 84% of the total water deliveries in FY 2024, with single-family residential (SFR) use accounting for 63% and multi-family residential (MFR) use accounting for 20%. Commercial and institutional accounts represent 8% and 3% of total use, respectively. Dedicated irrigation accounts used the smallest percentage of water at less than 1% of total production.

Based on a review of demand data and discussions with the District, the water demand data from FY 2021, prior to the recent drought, was assumed to be representative of existing demands. Total existing demand

⁵ The District's fiscal years span from 1 July of the prior calendar year to 30 June of the calendar year. As such, "FY 2015" represents 1 July 2014 to 30 June 2015, and so forth.

was estimated to be 2,853 acre-feet per year (AFY) for planning and modeling purposes, equal to the total average FY 2021 billing data plus the maximum construction water and water loss from the last five years (FY 2020 - FY 2024). To account for 32 new accounts added since FY 2021, demand was added for these accounts based on land use specific FY 2021 demand factors.

4.2 Water Demand Projections

The total projected future demand is estimated to be 3,477 AFY in FY 2045, consistent with demand projections presented in the District's 2020 Urban Water Management Plan (UWMP). As described in more detail in the 2020 UWMP, projected water demands were estimated by:

- Applying an estimated growth rate to the number of accounts within each water use sector based on projected population and employment growth rates,
- Identifying known planned developments within the District to verify that account growth projections consider all currently anticipated growth,
- Evaluating and selecting water demand factors for each water use sector based on review of recent average per account water use representing three scenarios (i.e., pre-drought conditions, post-drought conditions, and a partial drought rebound scenario),
- Estimating future passive savings using the Alliance for Water Efficiency (AWE) Water Conservation Tracking Tool (AWE model), and
- Calculating estimated future water demand that incorporates the anticipated account growth, water demand factors, and estimated future passive water savings.

4.2.1 Anticipated Development Projections

As discussed in Section 2.6, the District anticipates several large developments to be completed in the near-term future. Specifically, a 72-unit multi-family residential development on Verano Avenue across from Maxwell Farms Regional Park is anticipated to be complete by 2025 and the mixed-use Springs Specific Plan is planned to be completed over the next 50 years.⁶ EKI has estimated demands for these planned developments to more accurately allocate projected demands spatially within the District's service area. The Springs Specific Plan is anticipated to be fully completed beyond the District's planning horizon (2045), but EKI has included all the estimated water demands associated with the development in the future demands for conservatism.

For each of these developments, Table 4-6 summarizes the assumptions for increases in new development, assumed water demand factors, and projected increase in water demands for each land use type.

⁶ As discussed in Section 2.6, the District is considering the addition of the SDC but has not incorporated these into the water demand projections because they are in the infancy of the planning process.





Table 4-1
Current and Historical Potable Water Use and Population

	Potable Water	Service Area	Potable Water	Per Capita
Fiscal Year	Use	Population	Use	Potable Water Use
	(AF) (a)	(b)	(MG) (a)	(GPCD) (c)
1998	3,146	21,179	1,025	133
1999	3,518	21,432	1,146	147
2000	3,545	21,658	1,155	146
2001	3,526	21,853	1,149	144
2002	3,445	22,006	1,123	140
2003	3,394	22,237	1,106	136
2004	3,576	22,422	1,165	142
2005	3,298	22,913	1,075	129
2006	3,424	23,127	1,116	132
2007	3,484	23,239	1,135	134
2008	3,339	23,549	1,088	127
2009	3,039	23,515	990	115
2010	2,584	23,636	842	98
2011	2,733	23,717	890	103
2012	2,886	23,793	940	108
2013	3,042	23,801	991	114
2014	3,029	23,847	987	113
2015	2,528	23,874	824	95
2016	2,151	23,878	701	80
2017	2,415	23,927	787	90
2018	2,719	23,954	886	101
2019	2,445	22,912	797	95
2020	2,590	22,892	844	101
2021	2,810	22,912	916	109
2022	2,203	22,925	718	86
2023	2,043	22,955	666	79
2024	2,099	23,004	684	81

AF - acre-feet

GPCD - gallons per capita per day

MG - million gallons UWMP - Urban Water Management Plan

Notes:

- (a) Detailed historical and current water demand data from 2016 through 2024 are documented in Table 4-3 and Table 4-5.
- (b) Historical population from 1998 to 2007 is based on population estimates included in the 2015 UWMP. Population data from 2007 to 2018 are estimated following the 2015 UWMP assumption of 3.455 persons per service connection. Population data from 2019 to 2024 are estimated following the 2020 UWMP assumption of 3.3 persons per service connection.
- (c) Per capita potable water use is calculated by dividing the total annual potable water demand by service area population and the number of days in a year.



Figure 4-1 Current and Historical Potable Water Use and Population

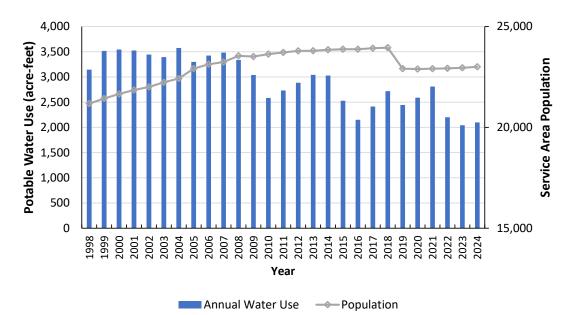


Figure 4-2 Current and Historical Per Capita Potable Water Use

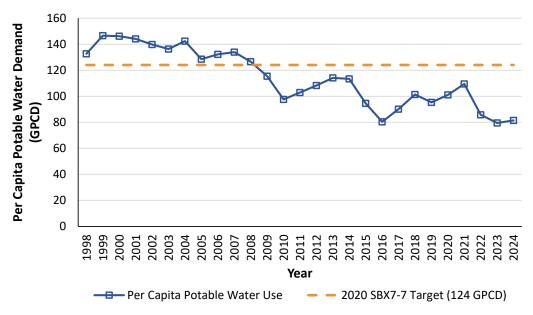




 Table 4-2

 Current and Historical Potable Water Production by Source

Potable Water Source		Annual Production (AF) (a)													
Folable water Source	FY 2011	FY 2012	FY 2013	FY 2014	FY 2015	FY 2016	FY 2017	FY 2018	FY 2019	FY 2020	FY 2021	FY 2022	FY 2023	FY 2024	
Imported Water															
SCWA	2,183	2,437	2,589	2,534	1,947	1,691	1,819	2,075	1,919	2,171	2,220	1,766	1,508	1,622	
Local Supply															
Donald	75	71	82	81	119	137	140	164	105	115	153	114	134	113	
Mountain Ave.	57	71	65	75	84	28	82	63	86	74	52	45	44	52	
Park Ave.	101	53	58	66	94	50	129	102	80	57	96	48	37	31	
Agua Caliente	99	87	92	123	135	121	111	166	133	128	168	115	121	99	
Verano	38														
Pedroncelli													0.02	0	
Craig													72	88	
Larbre	180	167	156	150	149	123	134	149	123	45	121	115	127	94	
Production Wells Total	550	449	453	495	581	459	596	644	526	419	589	437	535	477	
Total Water Production	2,733	2,886	3,042	3,029	2,528	2,151	2,415	2,719	2,445	2,590	2,810	2,203	2,043	2,099	
Percent Imported	80%	84%	85%	84%	77%	79%	75%	76%	78%	84%	79%	80%	74%	77%	
Percent Local Supply	20%	16%	15%	16%	23%	21%	25%	24%	22%	16%	21%	20%	26%	23%	

AF - acre-feet

FY - fiscal year

SCWA - Sonoma County Water Agency

Note:

(a) Production data for FY 2016 to FY 2018 provided by the District in October 2018. Production data for FY 2019 to FY 2024 provided by the District in September 2024.



Figure 4-3 Current and Historical Potable Water Production by Source

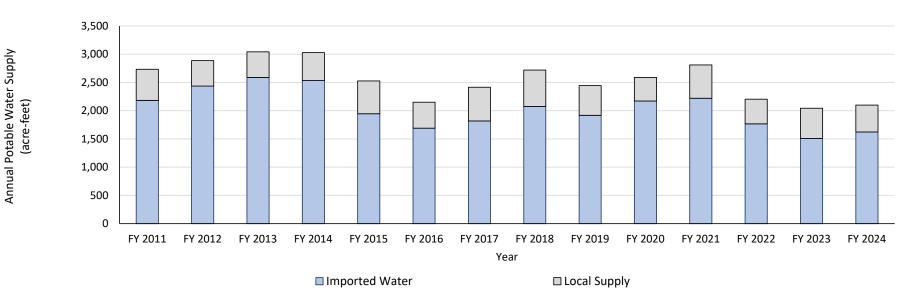




Table 4-3 Current and Historical Potable Water Use by Customer Category

		Potable Water Demand (AF) (a)														
Water Use Sector	FY 2010	FY 2011	FY 2012	FY 2013	FY 2014	FY 2015	FY 2016	FY 2017	FY 2018	FY 2019	FY 2020	FY 2021	FY 2022	FY 2023	FY 2024	
Single Family Residential	1,669	1,641	1,691	1,853	1,829	1,546	1,350	1,401	1,574	1,494	1,658	1,737	1,398	1,231	1,332	
Multi-Family Residential	476	469	505	531	516	457	411	438	484	468	513	507	443	422	426	
Commercial	178	179	180	185	182	176	167	169	176	172	170	146	143	157	172	
Institutional/Governmental	76	80	107	102	100	80	74	73	81	67	62	77	55	52	54	
Irrigation Multi-Family	24	29	30	34	44	32	17	24	42	37	35	39	21	17	21	
Irrigation Commercial	16	14	13	13	14	10	7	8	13	10	10	13	14	9	12	
Fire Services/Other	0.6	0.2	0.3	0.7	0.4	0.3	0.2	0.3	0.4	0.5	0.8	0.6	0.8	1.0	7.1	
Total Water Consumption	2,440	2,413	2,526	2,718	2,685	2,301	2,026	2,115	2,371	2,249	2,448	2,520	2,075	1,890	2,023	
Non-revenue Water (b)	6%	12%	12%	11%	11%	9%	6%	12%	13%	8%	5%	10%	6%	8%	4%	
Non-revenue water (b)	144	320	360	324	343	226	125	300	348	196	142	290	128	153	76	
Total Water Demand (c)	2,584	2,733	2,886	3,042	3,029	2,528	2,151	2,415	2,719	2,445	2,590	2,810	2,203	2,043	2,099	

AF - acre-feet

FY - fiscal year

Notes:

(a) FY 2016 - 2017 and FY 2018 - 2024 water use data were processed from water billing data provided by the District on 1 August 2018 and 30 October 2024, respectively.

(b) Non-revenue water includes water used for fire hydrant flushing and testing, for water main flushing, as well as distribution system water losses. This value is calculated as the difference between metered water consumption and total water production.

(c) Total water demand is the sum of metered water consumption and non-revenue water.

(d) Totals may not add exactly due to rounding.

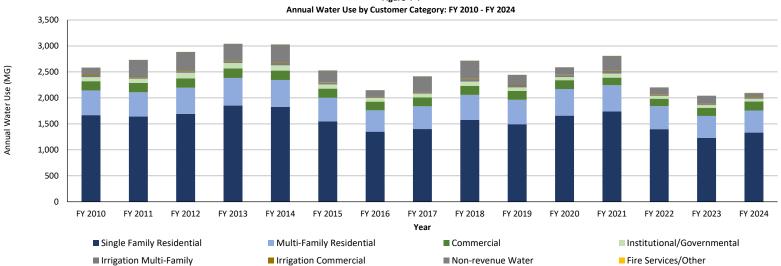


Figure 4-4

Section 4 Existing and Future Water Demands and Fire Flows



Figure 4-5 Percentage of Total Water Use by Customer Category: FY 2016 - FY 2024

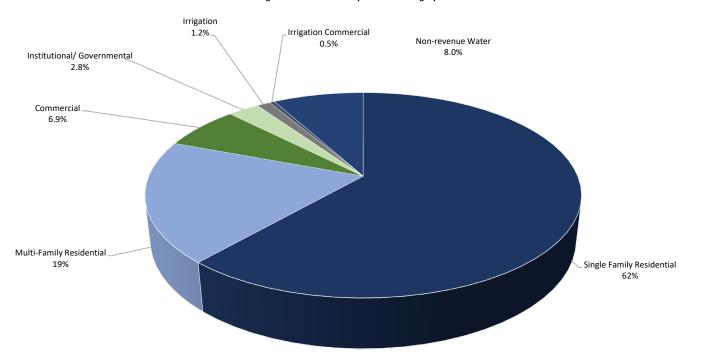




 Table 4-4

 Current and Historical Potable Water Use by Pressure Zone

Pressure Zone	Potable Water Demand (AF) (a)														
	FY 2010	FY 2011	FY 2012	FY 2013	FY 2014	FY 2015	FY 2016	FY 2017	FY 2018	FY 2019	FY 2020	FY 2021	FY 2022	FY 2023	FY 2024
1	1,678	1,656	1,710	1,856	1,822	1,580	1,391	1,453	1,632	1,549	1,663	1,698	1,428	1,325	1,393
1A	184	189	203	214	213	176	164	172	195	178	192	200	151	146	156
1B	208	200	213	229	230	190	165	171	190	195	209	211	172	150	163
1F	209	210	219	242	239	202	183	194	199	169	194	226	179	149	176
2A	1.7	2.1	2.5	2.2	2.1	1.8	1.4	1.2	1.2	1.5	2.1	1.8	1.5	0.9	1.0
2B	17	17	17	18	17	16	13	14	16	15	16	18	14	12	12
2D	93	89	91	97	104	85	70	71	85	81	91	95	78	66	69
2E	1.2	0.9	1.4	0.6	0.5	0.6	0.4	0.4	0.2	0.2	0.2	0.3	0.2	0.2	0.3
3D	10	10	11	12	13	12	9.0	8.7	10.1	10.8	12.4	12.5	9.8	8.2	8.7
3E	33	31	37	38	37	33	28	24	32	36	40	43	33	25	33
5E	0.9	1.7	0.3	0.4	0.3	0.3	0.3	0.5	0.3	0.2	0.2	0.2	0.2	0.3	0.5
SCWA (b)	3.1	2.8	4.8	3.9	5.3	4.7	11	3.9	3.5	3.7	4.5	4.7	5.0	5.8	5.1
Portable Hydrant Meters	1.7	4.1	16.8	5.5	2.7	0.4	0.3	2.1	5.2	10.4	24.1	9.0	2.3	1.3	3.9
Total Water Consumption	2,440	2,413	2,526	2,718	2,685	2,301	2,036	2,115	2,371	2,249	2,447	2,519	2,075	1,889	2,022
Non-revenue Water (c)	6%	12%	12%	11%	11%	9%	5%	12%	13%	8%	6%	10%	6%	8%	4%
Non-revenue Waler (C)	144	320	360	324	343	226	115	300	349	196	143	291	129	154	77
Total Water Demand (d)	2,584	2,733	2,886	3,042	3,029	2,528	2,151	2,415	2,719	2,445	2,590	2,810	2,203	2,043	2,099

AF - acre-feet

FY - fiscal year

SCWA - Sonoma County Water Agency

Notes:

(a) FY 2016 - 2017 and FY 2018 - 2024 water use data were processed from water billing data provided by the District on 1 August 2018 and 30 October 2024, respectively.

(b) SCWA serves these customers directly off of the aqueduct.

(c) Non-revenue water includes water used for fire hydrant flushing and testing, for water main flushing, as well as distribution system water losses. This value is calculated as the difference between metered water consumption and total water production.

(d) Total water demand is the sum of metered water consumption and non-revenue water.

(e) Totals may not add exactly due to rounding.

Section 4 Existing and Future Water Demands and Fire Flows



FY 2024

SCWA (b)

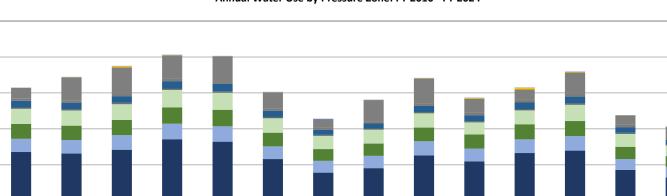


Figure 4-6 Annual Water Use by Pressure Zone: FY 2010 - FY 2024

1F 2A 🔳 2B 2D 2E 3E **5**E ■ Non-Revenue Water Figure 4-7 Percentage of Total Water Use by Pressure Zone: FY 2010 - FY 2024 PZ-2B PZ-3D PZ-3E PZ-2D PZ-2E 0.7% 0.5% 1.5% 3.6% 0.01% SCWA PZ-2A 0.2% 0.1% PZ-5E PZ-1F 0.01% 8.5% PZ-1B 8.3% PZ-1 PZ-1A 68.8% 7.9%

FY 2015 FY 2016

FY 2017

Year

FY 2018 FY 2019 FY 2020 FY 2021 FY 2022 FY 2023

Portable Hydrant Meters

Annual Water Use (MG)

1

3,500

3,000

2,500

2,000

1,500

1,000

500

0

1A

FY 2010

1B

FY 2011 FY 2012 FY 2013 FY 2014



Table 4-5

Current and Historical Potable Water Use by Pressure Zone and Customer Category

				Potable W	ater Dema	and (AF) (a	1)		
Sector	FY 2016	FY 2017		FY 2019				FY 2023	FY 2024
Pressure Zone 1									
Single Family Residential	879	921	1,039	995	1,098	1,128	924	820	874
Multi-Family Residential	322	336	369	349	369	381	339	327	325
Commercial	125	124	129	126	122	94	99	122	129
Institutional	46	46	51	39	37	50	36	34	32
Irr. Multi-Family Residential	13	20	34	32	31	34	19	15	18
Irr. Commercial	4.6	5.5	10.1	7.4	6.9	9.3	11.0	6.7	9.3
Fire Service/Other	0.2	0.1	0.0	0.0	0.1	0.1	0.2	0.3	6.2
Subtotal Pressure Zone 1	1,391	1,453	1,632	1,549	1,663	1,698	1,428	1,325	1,393
Pressure Zone 1A			_,						
Single Family Residential	117	121	137	126	139	146	113	105	110
Multi-Family Residential	42	44	49	46	48	50	36	38	43
Commercial	1.5	2.5	2.1	1.4	1.0	1.2	1.2	1.7	1.8
Irr. Multi-Family Residential	3.2	4.0	7.3	4.3	3.9	3.2	1.6	1.4	1.7
Subtotal Pressure Zone 1A	164	172	195	178	192	200	151	146	156
Pressure Zone 1B	107	172	155	170	152	200	101	110	150
Single Family Residential	105	110	122	118	130	136	105	93	104
Multi-Family Residential	28	29	32	37	45	41	42	35	33
Commercial	8.4	7.5	8.0	15	12	8.7	8.9	7.2	7.5
Institutional	23	23	27	23	20	23	15	13	17
Irr. Multi-Family Residential	0.4	0.4	0.3	0.4	0.2	1.0	0.1	0.3	0.8
Irr. Commercial	0.4	0.4	1	1.0	1.1	1.0	0.1	0.5	0.6
Fire Service/Other	0.00	0.03	0.02	0.02	0.02	0.02	0.01	0.7	0.0
Subtotal Pressure Zone 1B	165	171	190	195	209	211	172	150	163
Pressure Zone 1F	105	1/1	190	195	209	211	1/2	150	105
Single Family Residential	132	139	147	127	147	173	131	109	130
Multi-Family Residential	152	135	147	127	15	175	131	105	130
Commercial	31	33	32	26	28	33	32	25	30
Institutional	1.5	1.3	1.4	1.5	1.4	1.4	1.1	1.0	0.9
Irr. Multi-Family Residential	0.1	0.2	0.2	0.03	0.05	0.2	0.1	0.02	0.02
Irr. Commercial	2.1	2.2	2.4	1.9	2.1	2.2	2.0	1.3	1.9
Fire Service/Other	0.0	0.2	0.4	0.4	0.4	0.2	0.4	0.2	0.2
Subtotal Pressure Zone 1F	183	194	199	169	0.4 194	226	179	149	176
Pressure Zone 2A	185	194	199	109	194	220	179	149	170
Single Family Residential	1.4	1.2	1.2	1.5	2.1	1.8	1.5	0.9	1.0
Subtotal Pressure Zone 2A	1.4	1.2	1.2	1.5	2.1	1.8	1.5	0.9	1.0
	1.4	1.2	1.2	1.5	2.1	1.0	1.5	0.9	1.0
Pressure Zone 2B Single Family Residential	13	14	16	15	16	18	14	12	12
Single Failing Residential Subtotal Pressure Zone 2B									
	13	14	16	15	16	18	14	12	12
Pressure Zone 2D	60	60	02	70	00	02	75	C 4	67
Single Family Residential	68 2 5	68 2 5	82	78	88	92	75	64	67
Multi-Family Residential	2.5	2.5	3.5	2.7	2.8	3.1	2.6	2.4	2.2
Subtotal Pressure Zone 2D	70	71	85	81	91	95	78	66	69
Pressure Zone 2E		0.1	0.0	0.0	0.2	0.0	0.0	0.2	0.2
Single Family Residential	0.4	0.4	0.2	0.2	0.2	0.3	0.2	0.2	0.3
Subtotal Pressure Zone 2E	0.4	0.4	0.2	0.2	0.2	0.3	0.2	0.2	0.3
Pressure Zone 3D	7.0	7.0	0.1	0.5			0.0	7.0	7.0
Single Family Residential	7.8	7.8	9.1	9.6	11	11	8.9	7.2	7.8
Multi-Family Residential	1.2	0.8	1.1	1.3	1.4	1.0	0.9	1.1	1.0
Subtotal Pressure Zone 3D	9.0	8.7	10.1	10.8	12.4	12.5	9.8	8.2	8.7



Table 4-5 (Cont.)

Current and Historical Potable Water Use by Pressure Zone and Customer Category

Sector			F	otable W	ater Dema	nd (AF) (a	1)		
Sector	FY 2016	FY 2017	FY 2018	FY 2019	FY 2020	FY 2021	FY 2022	FY 2023	FY 2024
Pressure Zone 3E									
Single Family Residential	19	16	19	23	25	29	23	17	24
Multi-Family Residential	9	8	13	13	15	14	10	8	9
Subtotal Pressure Zone 3E	28	24	32	36	40	43	33	25	33
Pressure Zone 5E									
Single Family Residential	0.3	0.5	0.3	0.2	0.2	0.2	0.2	0.3	0.5
Subtotal Pressure Zone 5E	0.3	0.5	0.3	0.2	0.2	0.2	0.2	0.3	0.5
SCWA(c)									
SCWA - Single Family Residential (c)	7.7	1.0	0.8	0.7	1.3	1.7	1.3	1.5	1.4
SCWA - Institutional (c)	3.1	3.0	2.7	3.0	3.1	3.0	3.8	4.3	3.8
Subtotal SCWA (c)	11	3.9	3.5	3.7	4.5	4.7	5.0	5.8	5.1
Portable Hydrant Meters									
Hydrant Meter - Multi-Family Residential			0.9	6.9	17				
Hydrant Meter - Commercial	0	2	4.2	3.4	6.7	9.0	2.3	1.3	3.9
Subtotal of Other Uses	0.3	2.1	5.2	10.4	24.1	9.0	2.3	1.3	3.9
Total Water Consumption	2,036	2,115	2,371	2,249	2,447	2,519	2,075	1,889	2,022
Non-revenue Water	114.7	300.3	348.6	196.4	142.8	290.7	128.5	154.2	77.2
Total Water Demand (d)	2,151	2,415	2,719	2,445	2,590	2,810	2,203	2,043	2,099

Abbreviations:

Irr. - irrigation

AF- acre-feet FY - fiscal year

SCWA - Sonoma County Water Agency

Notes:

- (a) FY 2017 2018 and FY 2019 -2024 water use data were processed from water billing data provided by the District on 1 August 2018 and 30 October 2024, respectively.
- (b) Non-revenue water includes water used for fire hydrant flushing and testing, for water main flushing, as well as distribution system water losses. This value is calculated as the difference between metered water consumption and total water production.
- (c) SCWA serves these customers directly off of the aqueduct.
- (d) Total water demand is the sum of metered water consumption and non-revenue water.
- (e) Totals may not add exactly due to rounding.
- (f) Pressure Zone 4E is a pressure break between Pressure Zone 3E and 5E and does not have any service connections.

Section 4 Existing and Future Water Demands and Fire Flows



Table 4-6
Projected Water Demands for Anticipated New Development

Land Use Designation	Pressure Zone	Net Increase Developm		Water De	mand Factor Including Losses (b)	Net Increase in Water Demands (AF)				
810 W Agua Caliente Development (c)										
Single-Family Residential	1B	142	du	0.31	AFY/du (d)	43				
Multi-Family Residential	1B	220	du	0.13	AFY/du (d)	28				
Residential - Casitas	1B	120	du	0.15	AFY/du (e)	18				
Commercial - Hotel	1B	119	room	0.17	AFY/Room (f)	20				
Commercial - Assisted Living	1B	56	du	0.11	AFY/du (f)	6.4				
Commercial - Food Services	1B	21,270	sf	0.00011	AFY/sf (g)	2.2				
Commercial - Retail	1B	6,923	sf	0.00009	AFY/sf (g)	0.6				
Office	1B	23,100	sf	0.00007	AFY/sf (g)	1.6				
Outdoor Irrigation	Outdoor Irrigation 1B 844,020 sf (h)									
	150									
Verano Ave Multi-Family Residential	Development	(i)								
Multi-Family Residential	1	72	du	0.14	AFY/du (d)	10.1				
					Subtotal	10.1				
Springs Specific Plan					·					
Single-Family Residential	1	124	du	0.34	AFY/du (d)	42				
Multi-Family Residential	1	423	du	0.14	AFY/du (d)	59				
Work-Live/MF in Mixed Use	1	138	du	0.14	AFY/du (d)	19				
Commercial	1	167,029	sf	1.4	AFY/4,000 sf (j)	59				
Commercial - Hotel	1	120	Rooms	0.17	AFY/Room (f)	20				
Office	1	82,226	sf	1.40	AFY/4,000 sf (k)	29				
Recreation	1	26,648	sf	3.68	AFY/5,000 sf (I)	20				
	<u> </u>			•	Subtotal	249				
			TOTAL	NET INCREAS	SE IN WATER DEMANDS	409				

Abbreviations:

AF - acre-feet

DSS - Decision Support System

du - dwelling unit sf - square feet UMWP - Urban Water Management Plan WSA - Water Supply Assessment

Notes:

- (a) Net increases in new development were provided by the District based on current conservative estimates.
- (b) Factor of 1.12 applied to each demand factor to account for water loss.
- (c) The land use information and water demand factors for the 810 W Agua Caliente development are per the project's WSA completed in March 2024.
- (d) The single family and multi-family residential water demand factor are per the Districts 2020 UWMP.
- (e) Residential-Casitas water demand factor is from the August 2018 City of Redlands Impact Fees and Capacity Fees for Accessory Dwelling Units prepared by David Taussig & Associates.
- (f) Commercial-Hotel and Commercial-Assisted Living water demand factors are from the City of Ventura's April 2020 Final Water Demand Factor Study, City of Ventura, prepared by Wood Rodgers.
- (g) Commercial-Food Services, Commercial-Retail, and Office water demand factors are from the US Energy Information Administration's 2012 Commercial Buildings Energy Consumption Survey: Water Consumption in Large Buildings Summary.
- (h) Irrigation water demand is calculated per the MWELO MAWA. The calculations and assumptions are described in more detail in the project's WSA.
- (i) Verano Ave Multi-Family Residential Development project information provided by the District on 30 September 2024.
- (j) Commercial water demand factor is based on the DSS Model 2040 commercial per account demand, including passive savings, assuming 4,000 sf of commercial space per account consistent with the November 2018 Springs Specific Plan Projected Water Demands (De Novo, 2018).
- (k) Commercial water demand factor is based on the DSS Model 2040 commercial per account demand, including passive savings, assuming 4,000 sf of commercial space per account consistent with the November 2018 Springs Specific Plan Projected Water Demands (De Novo, 2018).
- Recreation water demand factor is based on the DSS Model 2040 institutional per account demand, including passive savings, assuming 5,000 sf of recreational space per account consistent with the November 2018 Springs Specific Plan Projected Water Demands (De Novo, 2018).

4.2.2 Total Projected Demands

Existing and future projected demands are summarized in Table 4-7 by pressure zone. The total projected future demand is consistent with the UWMP projections for 2045. Future demands for planned development projects were spatially allocated based on their location and land uses, and the remaining increase in demand was spread proportionally across the District.

			Future Demands (AFY)
Pressure Zone	Existing Demands (AFY) (a)	Projected Infill Growth (b)	Planned New Development	Total Future Demands (b) (c)
1	1,916	2,060	259	2,319
1A	224	241	-	241
1B	236	254	150	404
1F	257	277	-	277
2A	2.0	2	-	2.1
2B	20	21	-	21
2D	107	115	-	115
2E	0.29	0.31	-	0.31
3D	14	15	-	15
3E	48	52	-	52
4E	0.22	0.24	-	0.24
5E	5.3	6	-	5.7
SCWA	24.1	26	-	26.0
Total	2,853	3,042	409	3,477

Notes:

- (e) Equals the total average FY 2021 billing data by pressure zone plus maximum construction water and the max water loss from the last 5 years (FY 2020 - FY 2024). To account for the 32 new accounts added since 2021, demand was added for each account based on land use specific FY 2021 demand factor.
- (f) Projected infill growth and total future demands for each pressure zone are proportionate to the pressure zones' existing demands.
- (g) Total future demand of 3,477 AFY in 2045 per the District's 2020 UWMP.

4.3 Peak Demands

Peaking factors have not been updated as a part of the preparation of this update and are based on the peaking analysis performed for the 2019 WMP.

As a part of the 2019 WMP preparation, EKI reviewed the District's historical water consumption and production data and available SCADA flow and tank level data to identify the peaking factors for each pressure zone relative to average day demand (ADD) that best estimate maximum month demand (MMD), maximum day demands (MDD), and peak hour demands (PHD). A list of the sources of inflows, storage, and outflows for each zone are listed in Table 4-8. Based on the consumption data and calculated daily and hourly demands by zone, EKI developed pressure-zone-specific peaking factors. These peaking factors and key assumptions are listed in Table 4-9. Table 4-10 lists the projected existing and future peak demands by pressure zone based on these peaking factors and the average day demands.



The application of these peaking factors to assess system capacity and performance is described in Section 5.

4.4 Fire Flow Requirements

Fire protection for the District is provided by Sonoma Valley Fire and Rescue Authority and by the Glen Ellen Fire Department. The Sonoma Valley and Glen Ellen Fire Departments have indicated that a fire flow of 2,500 gpm should be provided in commercial and institutional areas. In residential areas, a fire flow of 1,000 gpm is required.⁷ These fire flows must be available for a minimum of two hours in conjunction with maximum day demand conditions while maintaining a minimum residual pressure of 20 pounds per square inch (psi) at all system nodes. The distribution of the required fire flows within the District are shown on Figure 4-8.

As discussed in the following sections, these flows and durations are used to establish supply capacity criteria, pipe sizing, and storage and supply capacity requirements.

⁷ In the 2007 Master Plan, a residential fire flow of 500 gpm was required and a fire flow of 1,000 gpm was desired. In light of the October 2017 Sonoma fires, this requirement has been updated to require a residential fire flow of 1,000 gpm.





 Table 4-8

 Sources of Inflow and Outflow and Storage by Pressure Zone

Pressure Zone	Sources of Flow In	Storage Tanks	Sources of Flow Out	Notes
1	<u>SCWA Turnouts^a:</u> Verano, Verano & Main, Verano & 5th, Boyes Blvd, Altimira, & Agua Caliente			
1A	<u>Wells:</u> Donald, Park Ave. Aqua Caliente, Verano, Pedroncelli, Craig, & Larbre	<u>Storage Tanks:</u> Bolli 1 & 2, Temelec 1 & 2, Donald	<u>Booster Pump Stations:</u> Agua Caliente & Donald	
2A	* <u>PRV:</u> Hanna Lower (only intended to open in emergency; not metered) ^b			Daily demands calculated together for
18	<u>SCWA Turnouts^a:</u> Madrone & Hanna * <u>PRV:</u> Eldridge (only intended to open in emergency; not metered) ^c	<u>Storage Tanks:</u> Hanna	* <u>PRV:</u> Hanna Lower (only intended to open in emergency; not metered) ^b	Pressure Zones 1, 1A, 2A, 1B, 2D, 3D, and 1F (only through October 2017) (see Note b, c, d) because the pressure zones were not
2D	<u>Wells:</u> Mountain Ave. <u>Booster Pump Stations:</u> Agua Caliente	<u>Storage Tanks:</u> Chestnut	<u>Booster Pump Stations:</u> Chestnut Isolation Valves between Pressure Zones 2D and 1 (typically closed) ^d	isolated.
3D ^e	Booster Pump Stations: Chestnut	Hydropneumatic: Chestnut ^f	None	
1F	<u>SCWA Turnouts^a:</u> Glen Ellen (6" & 4") & Trinity Oaks	Storage Tanks: Glen Ellen & Saddle	* <u>PRV:</u> Eldridge (only intended to open in emergency; not metered) ^c	Daily demands calculated as part of Pressure Zones 1, 1A, 2A, 1B, 2D, and 3D until November 2017 (see Note c).
2В	Booster Pump Stations: Donald	<u>Hydropneumatic:</u> Donald ^e	None	
2E	Booster Pump Stations: Sobre Vista Lower	<u>Storage Tanks</u> : Sobre Vista Lower	<u>Booster Pump Stations:</u> Sobre Vista Upper	Daily and hourly demands could not be calculated because flow and tank level data not sensitive enough for the low demands.
ЗE	Booster Pump Stations: Sobre Vista Upper	Storage Tanks: Sobre Vista Upper	<u>Booster Pump Stations:</u> Sonoma Mountain Lower	
4E	Booster Pump Stations: Sonoma Mountain Lower	<u>Storage Tanks</u> : Sonoma Mountain Lower	<u>Booster Pump Stations:</u> Sonoma Mountain Upper	Daily and hourly demands could not be calculated because flow and tank level data
5E	Booster Pump Stations: Sonoma Mountain Upper	<u>Storage Tanks</u> : Sonoma Mountain Upper	None	calculated because flow and tank level data not sensitive enough for the low demands.

(a) Only daily flow data available for SCWA turnouts. Therefore hourly demands could not be calculated for Pressure Zones 1, 1A, 2A, 1B, or 1F.

(b) A comparison of historical consumption data vs. calculated daily demand data for Pressure Zone 1B indicated that a significant amount of flow is being conveyed through the Hanna Lower PRV from Zone 1B to Zone 1 on a regular basis.

(c) A comparison of consumption data vs. calculated daily demand data for Pressure Zones 1F indicated that a significant amount of flow was being conveyed through the Eldridge PRV from Zone 1F to Zone 1B on a regular basis until October 2017. After October 2017, Pressure Zones 1F and 1B appear to be isolated (closed PRV), except for short periods of flow from 1F to 1B when the Hanna Turnout was not active.

(d) A comparison of historical consumption data vs. calculated daily demands for Pressure Zones 2D and 3D indicated that a significant amount of flow is being conveyed from Pressure Zone 2D to

(e) The District does not meter flow from the Chestnut Pump Station, so demands could not be calculated separately for Pressure Zone 3D.

(f) Storage in Chestnut and Donald Hydropneumatic Tanks not included in demand calculations.



Table 4-9 Summary of Peaking Factors by Pressure Zone

			Peaking Factors			
Pressure	[1]	[2]	[3]	[4]	[5]	
Zone	MMD to ADD (a)	MDD to MMD (b)	PHD to MDD (c)	MDD to ADD (d)	PHD to ADD (e)	
1	1.38			1.6	3.2	
1A	1.42			1.7	3.3	
1B	1.48	1.14	2.0	1.7	3.4	
2A	1.53			1.8	3.5	
2D	1.51			1.8	3.5	
3D	1.58	1.74	3.3	2.8	9.1	
1F	1.51	1.26	2.0	2.7	3.9	
2B	1.58	1.74	3.3	2.8	9.1	
2E	1.46			2.9	6.4	
3E	1.66	1.97	2.2	3.3	7.3	
5E	1.46			2.9	6.5	

- (a) Calculated based on average FY 2013 and FY 2014 consumption data.
- (b) Calculated based on daily demand calculations for FY2017 for Pressure Zones 2B and 3E and between November 2017 to August 2018 for Pressure Zones 1, 1A, 1B, 2A, 2D, and 3D and 1F (i.e., when zone 1F was isolated). Peaking daily demands exclude demands between 8 October 2017 and 16 October 2017 related to the October 2017 fires. Peaking Factor for Pressure Zone 3D assumed to equal the Pressure Zone 2B peaking factor because of similar land use.
- (c) Peak hour demands factors calculated for Pressure Zones 2B and 3E are based on the peak hour demands on the maximum demand day (6/18/2018 for Pressure Zone 2B and 7/21/2017 for Pressure Zone 3E). Calculated Pressure Zone 3E peaking factor assumed for Pressure Zones 2E and 5E. PHD to MDD peaking factor of 2.0 assumed for all other pressure zones; for comparison, a peaking factor of 1.67 was calculated for non-isolated Pressure Zones 2D and 3E. Peaking Factor for Pressure Zone 3D assumed to equal the Pressure Zone 2B peaking factor because of similar land use.
- (d) Equals Column [1] x Column [2], rounded up to the nearest tenth.
- (e) Equals Column [1] x Column [2] x Column [3], rounded up to the nearest tenth.

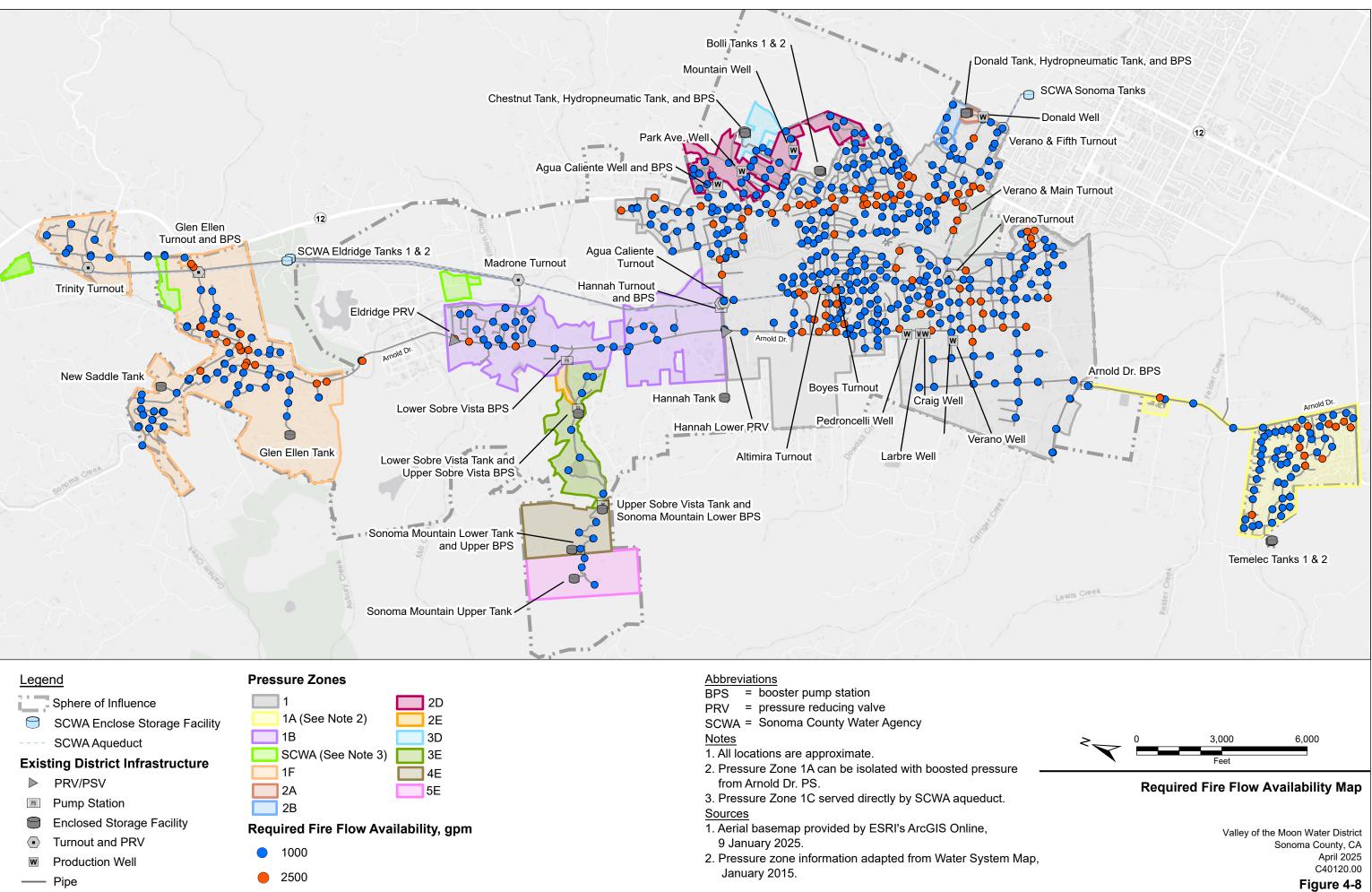


Table 4-10 Projected Existing and Future Peak Demands by Pressure Zone

Durante				Existing	Demands							Future I	Demands			
Pressure Zone (a)	Average Da	y Demand	laximum Mo	o <mark>nth Dem</mark> ar	Maximum D	ay Demand	Peak Hou	r Demand	Average Da	ay Demand	laximum Mo	onth Demar	Maximum D	ay Demand	Peak Hou	Demand
2011e (a)	1,000 gpd	gpm	1,000 gpd	gpm	1,000 gpd	gpm	1,000 gpd	gpm	1,000 gpd	gpm	1,000 gpd	gpm	1,000 gpd	gpm	1,000 gpd	gpm
1	1,711	1,188	2,359	1,638	2,737	1,901	5,474	3,801	2,070	1,438	2,856	1,983	3,312	2,300	6,625	4,601
1A	200	139	285	198	340	236	660	458	215	149	306	213	366	254	710	493
1B	211	146	312	216	358	249	717	498	361	250	533	370	613	426	1,226	851
1F	230	160	351	244	620	431	896	622	247	172	377	262	667	463	963	669
2A	1.8	1.2	2.7	1.9	3.2	2.2	6.2	4.3	1.9	1.3	2.9	2.0	3.4	2.4	6.6	4.6
2B	18	12	28	19	49	34	159	111	19	13	30	21	53	37	171	119
2D	95	66	143	99	171	118	332	230	102	71	154	107	183	127	357	248
2E	0.3	0.2	0.4	0.3	0.7	0.5	1.6	1.1	0.3	0.19	0.4	0.3	0.8	0.6	1.8	1.2
3D	12	8.7	18.2	12.6	35	24	114	79	13	9.3	19.5	13.6	38	26	122	85
3E	43	30	71	50	142	99	314	218	46	32	77	53	153	106	338	235
5E	0.2	0.1	0.3	0.2	0.6	0.4	1.3	0.9	5.1	3.5	7.4	5.1	14.7	10.2	32.9	22.9
Totals	2,521	1,751	3,570	2,479	4,457	3,095	8,675	6,024	3,081	2,139	4,363	3,030	5,404	3,752	10,554	7,329

Notes:

(a) SCWA Zone demands not included because these are supplied directly from the SCWA aqueduct and are not needed for distribution system master planning purposes.



5 WATER SUPPLY AND STORAGE CAPACITY EVALUATION

This section presents the supply and storage criteria and evaluates the District's water system storage and supply capacity to meet these criteria under existing and future demand scenarios described in Section 4.

5.1 Storage and Supply Capacity Criteria

EKI recommends continuing to use water supply and storage criteria that were updated in the *Evaluation* of Storage and Supply Requirements for Glen Ellen (EKI, 2021b), summarized below for clarity.

5.1.1 Water Supply and Pumping Capacity Requirements

As discussed in Section 4.3, peaking factors were developed to capture varying water use conditions. Higher water demand periods occur during summer months as compared to winter months when irrigation demands are higher. Over the course of the day, usage peaks in the early morning when people are preparing for their day and at night when people return home and begin irrigating their landscapes.

To meet varying demand conditions, EKI recommends a firm supply capacity through the combination of SWCA turnouts, groundwater wells, and booster pump stations in each pressure zone equal to the following:

- For pressure zones with storage to provide operational and fire storage by gravity (all but <u>Pressure Zones 1 & 2A, 2B, and 3D</u>): the sum of (1) maximum day demands of the pressure zones and the upper pressure zones that they supply and (2) the required supply to refill fire storage for the largest single fire flow in the zone (i.e., either residential = 120,000 gallon or commercial = 300,000 gallons) in 12 hours.
- For pressure zones that are only supplied pumped flows (Pressure Zones 2B and 3D): peak hour demands plus one residential fire; and
- For Pressure Zone 1 & 2A that are supplied peak flows by gravity from the aqueduct: the sum of peak hour demands for Pressure Zone 1 & 2A and maximum day demands for the upper pressure zones supplied by Pressure Zone 1.

Firm capacity is defined as follows:

- <u>For SCWA turnouts</u>: The largest turnout in the pressure zone is assumed out of service. For turnouts fed by gravity, capacity is assumed to equal the maximum flow capacity for Cla-Val model 90-01 PRVs.⁸ For the Hanna and Glen Ellen Turnouts, which require boosting from their associated pump stations under certain operational conditions (i.e., tank filling), the capacity is equal to the firm capacity of the pump station (i.e., with the largest pump out of service).
- <u>Groundwater wells:</u> The largest well in the pressure zone is assumed to be out of service.
- <u>Booster pump stations:</u> The largest single domestic pump is assumed out of service. For Donald Pump Station, the 300 gallons per minute (gpm) fire pump is assumed in service to supply fire flows to Pressure Zone 2B.

These criteria are recommended to ensure there is sufficient supply capacity distributed throughout the system to meet peak demand conditions. The District has also established a criterion to provide backup power for all pumping facilities (wells and booster pump stations).

⁸ Actual flow capacity of each PRV will vary based on system conditions.



5.1.2 Water Storage Capacity Requirements

Treated water storage capacity includes equalization storage, fire storage, and emergency storage. The following sections explain the requirement for each storage component in detail. Existing storage capacity is evaluated by pressure zone to meet the following criteria.

5.1.2.1 Operational Storage

As discussed in Section 5.1.1, the water supply criteria require firm water supply capacity to provide MDD in zones with operational storage. This storage is used to meet peak demand conditions. This storage volume is then refilled during low demand periods when water supply is greater than water demand.

The storage volume used to meet these high demand periods is called operational or equalization storage. Equalization storage is equal to 25% of MDD.

As discussed in Section 5.1.1, operational storage is not required for Zone 1 & 2A because the aqueduct directly supplies peak demands by gravity.

5.1.2.2 Emergency Storage

Emergency storage is required to supply demands during various emergencies, such as natural disasters, pipeline failures, treatment failures, power outages or pump station failure. No standard requirements exist for determining the appropriate amount of emergency storage, and each utility establishes these requirements based on their risk tolerance. The District's emergency storage requirement is 100% of ADD, and can be provided by a combination of sources, including:

- An upstream pressure zone as long as there is a reliable means to transfer the volume to that pressure zone during an emergency.
- Groundwater wells equipped with backup power equal to 18-hours of groundwater supply (approximately 0.25 MG).
- The SCWA Eldridge and Sonoma Tanks, accounting for a portion of the storage reserved for the City of Sonoma.⁹ Given that the SCWA Eldridge and Sonoma Tanks could serve the District by gravity under almost all foreseeable emergency conditions, it is appropriate to account for storage in the SCWA tanks in the District's storage evaluation. For conservatism, EKI recommends not accounting for storage in the Annadel Tanks based on the scenario in which a break occurs on the Aqueduct between Annadel Tanks and the District's service area.¹⁰

5.1.2.3 Fire Storage

As discussed in Section 4.4, fire flow and duration requirements were established for long-term planning purposes by the District and local fire departments. For the larger pressure zones with both residential and commercial land uses (Pressure Zones 1 & 2A, 1A, 1B, and 1F), EKI recommends fire storage to supply a concurrent commercial fire (2,500 gpm for 2 hours) and residential fire (1,000 gpm for 2 hours). This results in a fire storage requirement of 0.42 MG. This concurrent fire storage requirement is recommended to address vulnerabilities identified by the October 2017 Sonoma Fires, during which Glen Ellen Tank was drained to only 3 feet of water remaining and Saddle Tank was ultimately destroyed.

¹⁰ Updated from the 2019 WMP to reflect the change in storage requirements from the 2021 Evaluation of Storage and Supply Requirements for Glen Ellen.



⁹ Based on the 2006 Restructured Agreement for Water Supply (SCWA, 2006) the District can reasonably expect approximately 57% of the normal low storage volume in Eldridge and Sonoma Tanks in the event of a supply disruption, with the City of Sonoma retaining the remaining 43%.

For the smaller pressure zones with only residential land uses, EKI recommends fire storage to supply a single residential fire (1,000 gpm for 2 hours), resulting in a fire storage requirement of 0.12 MG.

Fire storage may be located in an upstream pressure zone as long as there is a reliable means to transfer the volume to that pressure zone during an emergency.

5.2 Supply and Pumping Evaluation

The District's existing firm supply capacity for each pressure zone was evaluated against the recommended criteria discussed in Section 5.1.1 under existing and projected future demand conditions. The results of this evaluation are presented in Table 5-1.

Table 5-1 indicates that Pressure Zones 2E, 3E, 2B, 3D, and 1F have supply capacity deficits under existing and future conditions. Pressure Zones 2B and 3D, which only are delivered pumped flows, do not have enough pumping capacity to meet the fire flow requirements. EKI recommends that dedicated fire pumps be added to each pump station that can meet the 1,000 gpm residential fire flow requirement.

Supply deficits (397 gpm and 369 gpm) are shown for Pressure Zone 1F for existing and future demand conditions. However, EKI conservatively did not include capacity for Trinity turnout in the zone supply capacity, which can supply peak demands by gravity in portions of the pressure zone. EKI recommends installation of a new 450 gpm BPS to deliver supply from Pressure Zone 1B to Pressure Zone 1F to meet the supply criteria. This new BPS also improve system resiliency and addresses fire flow concerns along Arnold Drive (see Section 7). Based on discussions with the District, we recommend installing the new Eldridge BPS and relocating the existing Eldridge PRV north of SDC. This location would allow for surface water supplies from SDC to be pumped to Pressure Zone 1F if these facilities are annexed by the District in the future.

As proposed by the District, EKI recommends that Pressure Zones 2E, which only has two service connections, and 3E be consolidated. Consolidation would consist of abandoning Sobre Vista Lower Tank, which is over 100-years-old and presents maintenance challenges, and Sobre Vista Upper Pump Station; connecting the two zones; installing individual service PRVs on the Pressure Zone 2E services; upgrading the main in zone 2E to handle the higher pressure; and upgrading Sobre Vista Lower Pump Station to be able to pump up to Sobre Vista Upper Tank. EKI recommends that as part of the Lower Sobre Vista BPS upgrades, the capacity be expanded to 290 gpm to meet the supply capacity requirements.

EKI also evaluated whether Arnold Drive BPS could be abandoned. During normal operating conditions, operating Arnold Drive BPS is not necessary; there is enough pressure in the system to fill the Temelec Tanks. However, as the District experienced in late 2018, if a portion of the SCWA Aqueduct is shutdown between Eldridge Tanks and Sonoma Tanks, Arnold Drive BPS is needed to move water south through the system and provide enough pressure to fill Temelec Tanks. Due to the existing poor condition and hazardous location along Arnold Drive, EKI recommends replacing Arnold Drive BPS and relocating the new BPS to Orange Avenue, between Solano Avenue and Arnold Drive.

In addition to the supply criteria discussed in Section 5.1.1, the SCWA Restructured Agreement established a goal for each of water contractors including the District to supply and maintain approximately 40% of its maximum month demand through local sources to mitigate against drought, emergencies, and temporary Transmission System outages. As discussed in Section 3.2.2, with the installation of the Pedroncelli, Craig, and new Park Well, the District's projected total groundwater capacity is approximately 1,150 gpm or approximately 46% of the existing maximum month demand and 38% of the projected future maximum month demand of 3,030 gpm (see Section 4.4). Based on available design flow rates and production estimates, the District will meet local supply requirements for existing demands but may need



to increase its local supply slightly as demands increase in the future to meet the 40% maximum month demand supply goal.

These recommended supply improvements are discussed in more detail in Section 8.





 Table 5-1

 Water Supply Capacity Requirements by Pressure Zone

	Exist	ing Firm Supp	oly Capacity (gpm)		Existing Re	quirements	Future Rec	quirements
Pressure Zone(s)	SCWA Turnouts (a)	Wells (b)	Booster Pump Stations (c)	Total	Required Supply Criteria (d)	Required Firm Supply Capacity (gpm)	Firm Supply Capacity Surplus (Deficit) (gpm)	Required Firm Supply Capacity (gpm)	Firm Supply Capacity Surplus (Deficit) (gpm)
1 & 2A	11,600	815	-	12,415	1 & 2A PHD + 1A, 2B, 2D, 3D MDD	4,218	8,197	5,049	7,366
1A	-	-	500	500	1A MDD + Tank Fill	403	97	421	79
2В	-	-	450	450	2B PHD + FF	1,111	-661	1,119	-669
2D	-	-	350	350	2D & 3D MDD	309	41	320	30
3D	-	-	100	100	3D PHD + FF	1,079	-979	1,085	-985
1F	450	-	-	450	1F MDD + Tank Fill	847	-397	880	-430
1B	800	-	-	800	1B, 2E, 3E, 5E MDD + Tank Fill	765	35	959	-159
2E	-	-	130	130	2E, 3E, 5E MDD + Tank Fill	266	-136	284	-154
ЗE	-	-	100	100	3E, 5E MDD + Tank Fill	266	-166	283	-183
5E	-	-	17	17	5E MDD	0.4	17	10.2	7

(a) Firm turnout capacity defined as the maximum rated flow capacity of each turnout with one turnout in each pressure zone offline. For gravity fed turnouts maximum capacity is equal to PRV flow capacity. For pumped turnouts, Hannah and Glen Ellen, capacity is equal to the firm capacity of the associated booster pump station.

- (b) Firm well capacity defined as the total well capacity with the largest well offline.
- (c) Firm pumping capacity defined as the total capacity of all pumps minus the capacity of one domestic pump.
- (d) Required supply criteria for each pressure zone is further described in Section 5.1.1.

5.3 Storage Evaluation

The District's required storage capacity is composed of equalization, fire, and emergency storage described in Section 5.1.2. The District currently has 5.5 MG of storage between its 13 tanks and an additional 7.4 MG of SCWA storage capacity. Table 5-2 provides a summary of existing and future storage requirements and capacity by pressure zone.

Based on the system-wide evaluation presented in Table 5-2, a significant 3.87 MG storage surplus exists. With the District's current storage facilities and accounting for SCWA storage and groundwater supplies, existing emergency storage ranges from 3.5 to 4.2 days of ADD and future available emergency storage is projected to range between 3.2 to 3.7 days of ADD depending on the demand assumptions. This represents a robust volume of emergency storage that should allow for the District and SCWA under most scenarios to address any supply disruption for the storage is depleted.

Pressure Zone 2E, which currently cannot receive transfer of storage from Pressure Zones 3E, does not have enough fire flow storage. This deficiency will be resolved with the proposed Pressure Zones 2E and 3E consolidation, described in Section 5.2. As part of this project, EKI recommends installing a PRV between Pressure Zones 2E and 1B, so that storage in Sobre Vista Upper Tank can be transferred to the lower zones, if needed.

These recommended storage improvements are discussed in more detail in Section 8.



Section 5 Water Supply and Storage Capacity Evaluation



Table 5-2 Water Storage Capacity Requirements by Pressure Zone

		Available S	torage Capa	city (a)		Existing Re	quirements					Futu	ire Requireme	nts	
Pressure Zone(s)	District Storage (1,000 gal)	SCWA Storage (1,000 gal)	GW Credit (1,000 gal)	Total Available Storage (1,000 gal)	Notes	Operational Storage (1,000 gal) (b)	Fire Storage (1,000 gal) (c)	Emergency Storage (1,000 gal) (d)	Total Required Storage (1,000 gal)	Storage Capacity Surplus (Deficit) (1,000 gal)	Operational Storage (1,000 gal) (b)	Fire Storage (1,000 gal) (c)	Emergency Storage (1,000 gal) (d)	Total Required Storage (1,000 gal)	Storage Capacity Surplus (Deficit) (1,000 gal)
1F	650	2,908	-	3,558	Can deliver storage to 1 by gravity and can receive storage from 1B (pumped)	155	420	230	805	2,753	167	420	247	834	2,724
2D & 3D	320	-	-	320	Can deliver storage to 1 by gravity	51	120	107	279	41	55	120	115	291	29
2B	200	-	-	200	Can deliver storage to 1 and 2A by gravity	12	120	18	150	50	13	120	19	152	48
1A	1,200	-	-	1,200	Typically operates as single zone with 1 and 2A; can deliver storage to 1 and 2A by gravity	85	420	200	705	495	91	420	215	727	473
4E & 5E	54	-	-	54		0.14	0 (e)	0.20	0.34	54	3.67	0 (e)	5.07	8.74	45
2E & 3E	207.5	-	-	208	Zones 2E and 3E consolidated and SV Lower Tank abandoned; PRV added to deliver storage to 1B by gravity. Note that the Sobre Vista Lower Tank volume has been removed from the total capacity.	36	120	43	199	8	38	120	47	205	2
1B	2,000	-	-	2,000	Can receive storage from 1F, 2E-5E, and 1 (pumped), and deliver to 1 by gravity	90	420	211	720	1,280	153	420	361	934	1,066
1 & 2A	800	4,471	253	5,524	Can receive storage from 1A, 1B, 1F (through 1B), 2E-5E (through 1B), 2B, and 2D by gravity.	685	420	1,712	2,817	2707	829	420	2,072	3,321	2,203
TOTAL	5,432	7,379	253	13,063		1,114	2,040	2,521	5,675	4,635	1,351	2,040	3,081	5,638	3,868

Notes:

(a) Refer to Table 3-6 for existing District storage. Note that the Sobre Vista Lower Tank capacity has been removed based on the recommendation to abandon the tank. For SCWA Storage, per the 2005 Memorandum of Understanding Regarding Water Transmission System Capacity Allocation During Temporary Impairment, Table 1 indicates Valley of the Moon Water District is entitled to 4.9 MGD and Sonoma is entitled to 3.8 MGD. The 2006 Restructured Agreement for Water Supply presents an entitlement of 8.5 MGD for the District and 6.3 MGD for Sonoma. Thus, it is conservatively assumed that the District can expect approximately 50% of the available storage in the Eldridge (8MG) and Sonoma Tanks (10MG) which equates to 6.8 MG. The storage available in the SCWA tanks is based on the normal low level as seen in SCADA data from 2016 to 2018. Storage from the Sonoma Tanks could be fed to Zone 1 or 1B, but are shown here providing storage for only Pressure Zone 1.

(b) Operational storage equal to 25% of max day demands.

(c) Fire storage volume for Pressure Zones 1 & 2A, 1A, 1B, and 1F equal to volume require to supply a concurrent commercial fire (2,500 gpm for 2 hours) and residential fire (1,000 gpm 2 hours). Fire storage volume for other pressure zones equal to volume required to supply a single residential fire (1,000 gpm 2 hours).

(d) Emergency storage volume equal to 100% of ADD.

(e) Fire storage volume requirement for the Sonoma Mountain Homestead area provided by a private irrigation system.

(f) Operational storage for Pressure Zones 1 and 2A is not required because Aqueduct provides peak demands by gravity. While this is also normally true for Pressure Zones 1A and 1B, operational storage is required because pumping is still required under certain operational conditions.

5.4 Summary of Recommended Supply and Storage Facility Improvements

EKI recommends the following improvements to address projected supply and storage deficits:

- Addition of dedicated fire pumps at Donald Pump Station and Chestnut Pump Station with enough capacity to meet the fire flow requirements.
- Installation of a new 450-gpm pump station to supply flow from Pressure Zone 1B to Pressure Zone 1F.
- Consolidation of Pressure Zones 2E and 3E by abandoning Sobre Vista Lower Tank and Sobre Vista Upper Pump Station, replacing the Sobre Vista Lower Pump Station pumps with higher head, 290-gpm pumps.
- Installation of a new developer-funded turnout to feed Pressure Zone 1B as a part of the 810 W. Agua Caliente development.

In addition to these capacity-related improvements, EKI has evaluated the following proposed improvements to simplify operations and improve system resiliency and redundancy:

- Installation of a new PRV between Pressure Zones 3E/2E and 1B to be able to transfer surplus storage to lower zones.
- Replacement and relocation of the Eldridge PRV in conjunction with the new Zone 1F pump station north of SDC.
- Replacement of Arnold Drive Pump Station based on the existing pump station condition and access restrictions.
- Addition of backup generators to remaining critical wells and pump stations currently without backup power.
- Installation of flow meters at each of the SCWA turnout PRVs and SCADA integration.

Table 5-3 and Table 5-4 show that with the improvements discussed above, the District will address its projected supply and storage deficits. These recommended improvements are further investigated based on hydraulic modeling analysis in Section 7 and are integrated into the recommended CIP as discussed in Section 8.



Section 5 Water Supply and Storage Capacity Evaluation



Table 5-3
Water Supply Capacity Requirements by Pressure Zone with Recommended Improvements

		Proposed F	irm Supply Capacity (gpm)		Future Requirements		
Pressure Zone(s)	SCWA Turnouts (a)	Wells (b)	Booster Pump Stations (c)	Total	Required Supply Criteria (d)	Required Firm Supply Capacity (gpm)	Firm Supply Capacity Surplus (Deficit) (gpm)
1 & 2A	11,600	555	-	12,155	1 & 2A PHD + 1A, 2B, 2D, 3D MDD	5,049	7,106
1A	-	-	500	500	1A MDD + Tank Fill	421	79
2В	-	-	1125 (e)	1,125	2B PHD + FF	1,119	6
2D	-	-	350	350	2D & 3D MDD	320	30
3D	-	-	1100 (f)	1,100	3D PHD + FF	1,085	15
1F	450	-	450 (g)	900	1F MDD + Tank Fill	880	20
1B	800	-	-	800	1B, 2E/3E, 5E MDD + Tank Fill	959	-159 (h)
2E/3E	-	-	290 (i)	290	2E/3E, 5E MDD + Tank Fill	284	6
5E	-	-	17	17	5E MDD	10.2	7

Notes:

- (a) Firm turnout capacity defined as the maximum rated flow capacity of each turnout with one turnout in each pressure zone offline. For gravity fed turnouts maximum capacity is equal to PRV flow capacity. For pumped turnouts, Hannah and Glen Ellen, capacity is equal to the firm capacity of the associated booster pump station.
- (b) Firm well capacity defined as the total well capacity with the largest well offline.
- (c) Firm pumping capacity defined as the total capacity of all pumps minus the capacity of one domestic pump.
- (d) Required supply criteria for each pressure zone is further described in Section 5.1.1.
- (e) Proposed Donald BPS upgrades include two 125-gpm domestic pumps and one 1,000-gpm fire pump.
- (f) Proposed Chestnut BPS upgrades include two 100-gpm domestic pumps and one 1,000-gpm fire pump.
- (g) Capacity accounts for proposed 450-gpm Eldridge BPS.
- (h) Deficiency to be addressed by a new turnout installed as a part of the 810 W. Agua Caliente Development.
- (i) Capacity accounts for proposed Sobre Vista Lower BPS upgrades, including two 290-gpm pumps.



Table 5	5-4
---------	-----

Water Storage Capacity Requirements by Pressure Zone with Recommended Improvements

		Storage Capacity (a) (b)	Future Requirements						
Pressure Zone(s)	Existing (1,000 gal)	Notes	Operational Storage (1,000 gal)	Fire Storage (1,000 gal)	Emergency Storage (1,000 gal)	Total Required Storage (1,000 gal)	Available Surplus from Upstream Zone (1,000 gal)	Storage Capacity Surplus (Deficit) (1,000 gal)	
1F	3,558	Can deliver storage to 1B by gravity	167	420	247	834	-	2,724	
2D & 3D	320	Can deliver storage to 1 by gravity	55	120	115	291	-	29	
2В	200	Can deliver storage to 1 and 2A by gravity	13	120	19	152	-	48	
1A	1,200	Typically operates as single zone with 1 and 2A; can deliver storage to 1 and 2A by gravity	91	420	215	727	-	473	
4E & 5E	54		3.67	0 (c)	5.07	8.74	-	45	
2E & 3E	207.5	Zones 2E and 3E consolidated and SV Lower Tank abandoned; PRV added to deliver storage to 1B by gravity	38	120	47	205	0	2	
1B	2,000	Can receive storage from 1F and 2E-5E and deliver to 1 by gravity	153	420	361	934	2,726	3,792	
1 & 2A	5,524	Can receive storage from 1A, 1B, 1F (through 1B), 2E-5E (through 1B), 2B, and 2D by gravity.	0 (d)	420	2,072	2,492	4,343	7,375	
TOTAL	13,063	· · · · · · · · · · · · · · · · · · ·							

Notes:

(a) Proposed storage capacity includes consolidation of Pressure Zones 2E and 3E and abandonment of Sobre Vista Lower Tank, and installation of new PRVs between 5E and 4E, 4E and 3E/2E, and 3E/2E and 1B.

(b) = denotes improvements

(c) Fire storage volume requirement for the Sonoma Mountain Homestead area provided by a private irrigation system.

(d) Operational storage for Pressure Zones 1 and 2A is not required because Aqueduct provides peak demands by gravity. While this is also normally true for Pressure Zones 1A and 1B, operational storage is required because pumping is still required under certain operational conditions.

6 WATER DISTRIBUTION SYSTEM PERFORMANCE AND SIZING CRITERIA

This section develops recommended water distribution system performance and sizing criteria for water distribution pipelines to be used when evaluating the District's water system under existing and future modeling scenarios described in Section 7.1. These criteria are summarized in Table 6-1.

6.1 Distribution System Pressures

The distribution system's ability to maintain adequate system pressures is the primary indicator of acceptable system performance. Under normal operating conditions, the distribution system is required to maintain a minimum pressure of 35 psi at all service connections based on District standards. A maximum pressure of 80 psi is required at all service connections where there are not individual service PRVs installed. It is understood that the District has agreements with residents at certain far reaches of the distribution system where the system frequently operates below 35 psi. In these locations, some residents have installed and maintain small booster pumps on their service connections to increase pressure as-needed. Further, while 35 psi is acceptable for most uses, fire sprinkler systems may need upwards of 50 psi to function optimally. When new services include fire sprinkler systems, the District reviews the plans and documents whether booster pumps have been deemed necessary.

As discussed in Section 4.4, the system must be able to maintain a minimum of 20 psi throughout the system under fire flow conditions to comply with California Code of Regulations, Title 22 (Title 22) requirements.

6.2 Water Transmission and Distribution Pipeline Sizing Criteria

The following pipeline velocity and head loss criteria are used for sizing new transmission and distribution pipelines. However, when evaluating the existing system, velocity and head loss criteria are secondary to the system pressure criteria (Section 6.1).

For example, if system pressures are satisfied under PHD and MDD plus fire flow (FF) conditions, an existing pipe that exceeds maximum velocity or head loss criteria are not necessarily indicative of a problem that requires system improvements. Any identified exceedances have been reviewed on a caseby-case basis to determine if they are influencing any deficient system pressures or if improving these pipes to meet velocity or head loss criteria would benefit the water movement within the system. In certain cases, upsizing deficient upstream piping near supply sources where flow and headloss are greatest can effectively address multiple downstream pressure deficiencies.

6.3 Velocity Criteria

The following velocity criteria, in conjunction with head loss criteria described below, are recommended for sizing of new water mains:

- PHD conditions: Maximum velocity of 6 feet per second (fps) for all mains
- MDD plus fire flow conditions: Maximum velocity of 10 fps for all mains

As discussed above, for existing infrastructure these criteria are secondary to pressure criteria and are evaluated to identify potential bottlenecks in the system that could be upsized to address pressure deficiencies.



6.4 Headloss Criterion

In addition to velocity criteria, the following head loss criterion must also be met for sizing of new water mains:

• PHD conditions: Maximum head loss of 7 feet per 1,000 feet of pipe (ft/k-ft)

For existing pipelines this criterion is used to identify bottlenecks in the system that if upsized could relieve downstream pressure to meet pressure criteria and improve connectivity of major supply sources and storage facilities to outlying areas.





Table 6

Summary of Recommended Potable Water System Performance and Operational Criteria

Component	Criteria				
Fire Flow Requirements					
Single Family Residential	1,000 gpm for 2 hours.				
General Commercial and Office	2,500 gpm for 2 hours.				
Distribution System Pressure Requirements					
Maximum Pressure	80 psi at customer service connections without PRVs on service laterals				
Minimum Pressure - Normal Operating Conditions (a)(b)	35 psi at customer service connections excluding fire flow				
Minimum Pressure - Max Day Plus Fire Flow Conditions	20 psi (Title 22 requirement for minimum allowable pressure)				
Recommended Pipeline Sizing Criteria					
Maximum Valacity (Secondary)	6 ft/s, all system mains, peak hour demand				
Maximum Velocity (Secondary)	10 ft/s, all system mains, maximum day plus fire flow				
Maximum Head Loss (Secondary)	7 ft per 1,000 ft, peak hour demand				
Hazen Williams "C" Factor	New piping = 140				
	Existing system piping = 80 - 150 per model calibration				
	General: 8-inch diameter or larger				
	Cul-de-sacs and dead end runs: 6-inch diameter acceptable within cul- de-sacs				
New Distribution Main Diameter	and dead end runs of less than 500 feet where future extensions will not occur				
	and no fire hydrants are located.				
	Commercial: 10-inch diameter or larger.				

Notes:

(a) Title 22 CCR Section 64602 requires that water distribution systems maintain a minimum of 40 psi in each expansion of the distribution system that expands the existing system service connections by more than 20 percent. No developments of this magnitude are currently planned.

The District maintains individual agreements with customers where the minimum pressure cannot be maintained due to elevation.

(b) Customers may need to provide booster pumps to increase pressure for fire suppression systems.

7 WATER DISTRIBUTION SYSTEM MODELING EVALUATION

The following sections describe the modeling scenarios, approach, and results.

7.1 Modeling Scenarios

The District's water system was evaluated under the existing demand and projected future demand condition discussed in Section 4. As a part of the streamlined approach to this plan update, demands were not spatially re-allocated in the hydraulic model based on billing data. Instead, for the existing scenario the 2019 allocations have been scaled proportionally across the District and for the future scenario the demands for the planned new development were spatially allocated with the remaining demand scaled proportionally across the District.

These scenarios were evaluated under the modeling simulations described further described in Section 7 to identify existing and projected future capacity deficiencies.

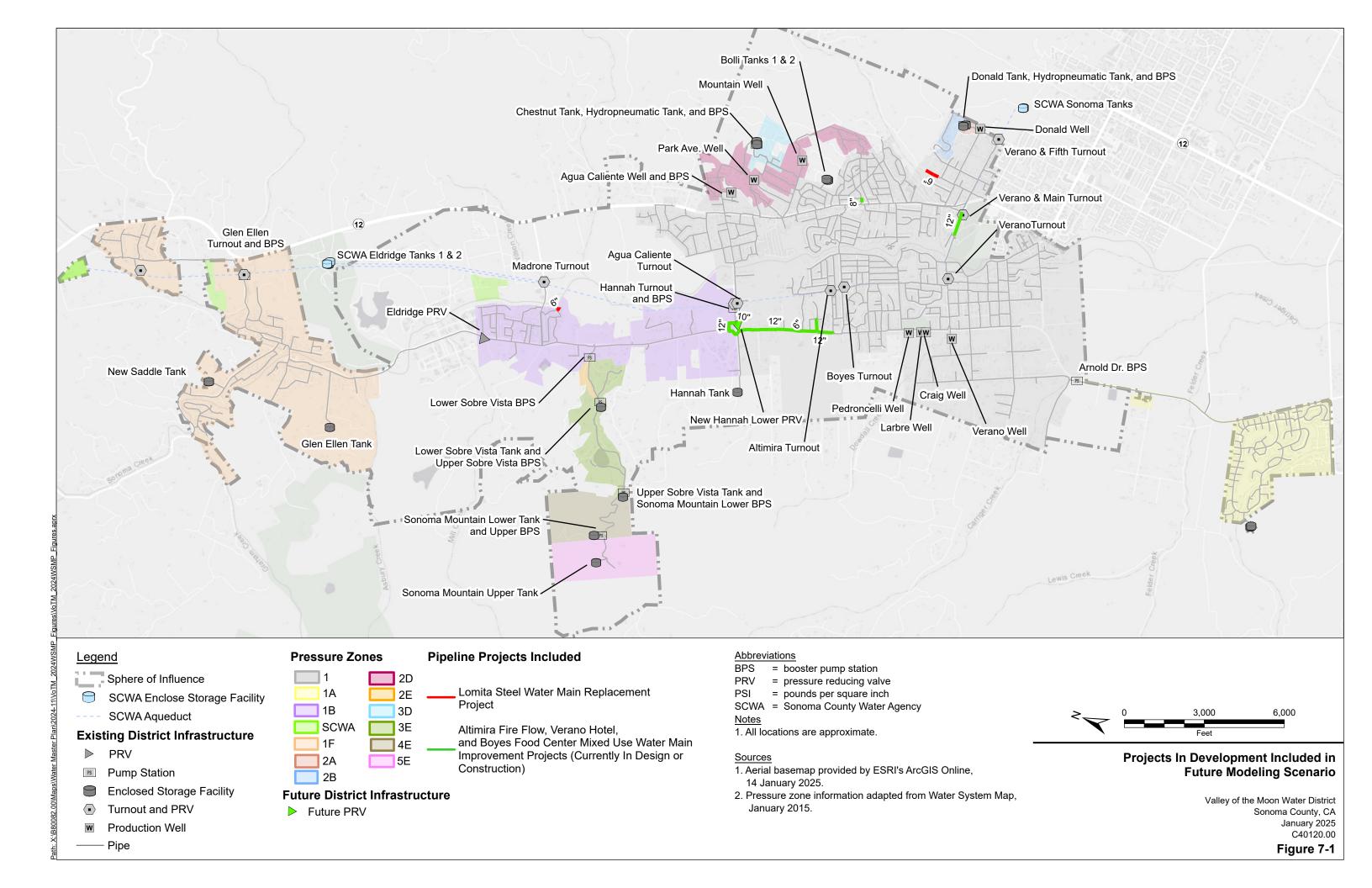
Several updates were made to the existing scenario, including:

- The addition of Saddle Tank and incorporation of other CIP and development projects completed since the 2019 WMP, as discussed in Section 3;
- Scaling of previously allocated demands and allocation of new development demands to reflect changes in demands, as discussed in Section 4.2.2; and
- Adjustments to PRV set points per District operators logs dated 20 September 2024 as shown in Table 3-5;

As shown in Figure 7-1, the future scenario includes upcoming CIP and development projects currently in design or construction, including:

- The Brookview and Riddle Road Easement Steel Replacement Project;
- Altimira Fire Flow Improvement Project;
- Verano Hotel Frontage Public Water Main Improvements;
- 18661 Lomita Avenue New Water Main Project; and,
- Boyes Food Center Mixed Use Development Water Main Improvement Project.





7.2 Modeling Approach

To evaluate distribution system performance against performance criteria, EKI conducted steady-state model simulations of (1) PHD and (2) MDD+FF for both the Existing, Future, and Future with CIPs Scenarios.

As a part of this modeling analysis, the District identified three areas in the system which could benefit from the installation of new pressure regulating stations to reduce pressures. These three areas have been included in the Future with CIPs scenario to ensure no unintended effects occurred with the re-zoning of these portions of the system.

7.2.1 PHD Simulations

The followings operating conditions were assumed under the PHD model runs to represent normal "worst-case" operating conditions:

- All groundwater wells were out of service;¹¹
- Donald and Chestnut BPSs operating at firm domestic capacity (i.e., with the largest pump out of service) with all fire pumps offline;
- All other BPSs offline;
- All PRVs set at the recommended PRV settings listed in Table 3-5;
- All tanks (including Eldridge and Sonoma SCWA Tanks) filled to their normal low levels;
- Eldridge Tank and Sonoma Tank filling at 2,700 gpm and 500 gpm, respectively, which represent the average fill rates according to an analysis of available SCADA data; and
- The hydraulic grade level in the SCWA Aqueduct as it enters the District set at approximately the average level.

Results from these simulations provided information on junction pressures and pipeline head loss and velocity under PHD conditions.

7.2.2 MDD + FF Simulations

The MDD + FF scenarios were run under the same operational conditions as the PHD simulations with the following exceptions:

- Fire pumps are available as-needed; and
- BPSs controls were set such that pumps switched on (up to firm capacity) when upstream pressures dropped below normal levels, representing the District's ability to manually turn pumps on if needed during an emergency.

The fire flow simulations determine the fire flow availability at each hydrant while maintaining a minimum of 20 psi everywhere in the system. These results were compared to required fire flows to determine which hydrants do not meet the required criteria. To address observed fire flow deficiencies, additional fire flow analyses were conducted under MDD+FF conditions on a case by case basis by manually applying

¹¹ As shown in Appendix C of the 2019 WMP, Figure C-1, identified peak hour and fire flow deficiencies are largely the same with groundwater wells in service, indicating the wells have minimal influence on the fire flow availability. Groundwater wells have been assumed to be offline to best represent the worst-case scenario.



fire demands on individual hydrant nodes to assess pipeline head loss and velocity under fire flow conditions.

7.3 Existing Scenario Evaluation

Existing scenario model simulations were evaluated to identify existing system deficiencies, as are discussed in the following sections.

7.3.1 PHD Results

Model results indicate that the existing system cannot meet the minimum pressure criteria of 35 psi system-wide under PHD conditions. Modeled system pressures at service connections ranged from approximately 27 psi to 138 psi, as shown on Figure 7-2. The locations of the lowest pressures were observed at similar locations to the 2019 analysis, including high elevation service connections near the Chestnut Tank, the Sobre Vista Upper Tank, Glen Ellen Tank, and prior to the Agua Caliente Well and pump station along East Aqua Caliente Road. Areas east of Agua Caliente Turnout have been observed to have higher pressures than the 2019 WMP due to modifications to the District's PRV settings, however, a few locations are still below 35 psi.

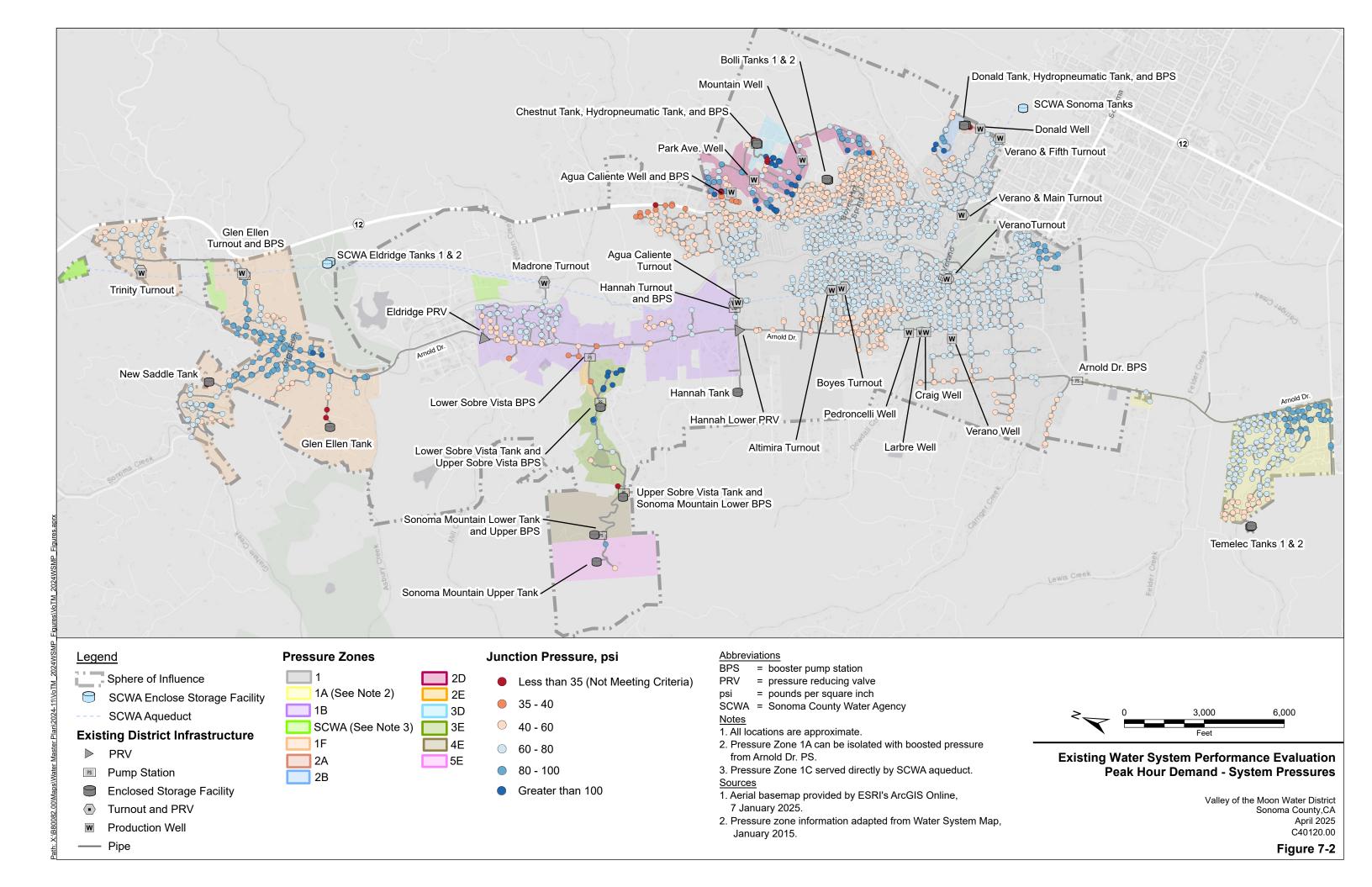
Analysis of pipeline head losses and velocities indicates that the system is generally able to meet the required criteria. The highest headloss of 28 ft/k-ft is experienced on the 4-inch main on Railroad Avenue between El Dorado Drive and Boyes Boulevard and the 4-inch main on Los Banos Drive between Altimira Circle and Boyes Boulevard, which exceeds the criteria of 7 ft/k-ft. Headloss is a secondary criterion, this section of pipeline likely does not need replacement based on modifications to the system pressure zones discussed further in Section 7.6.

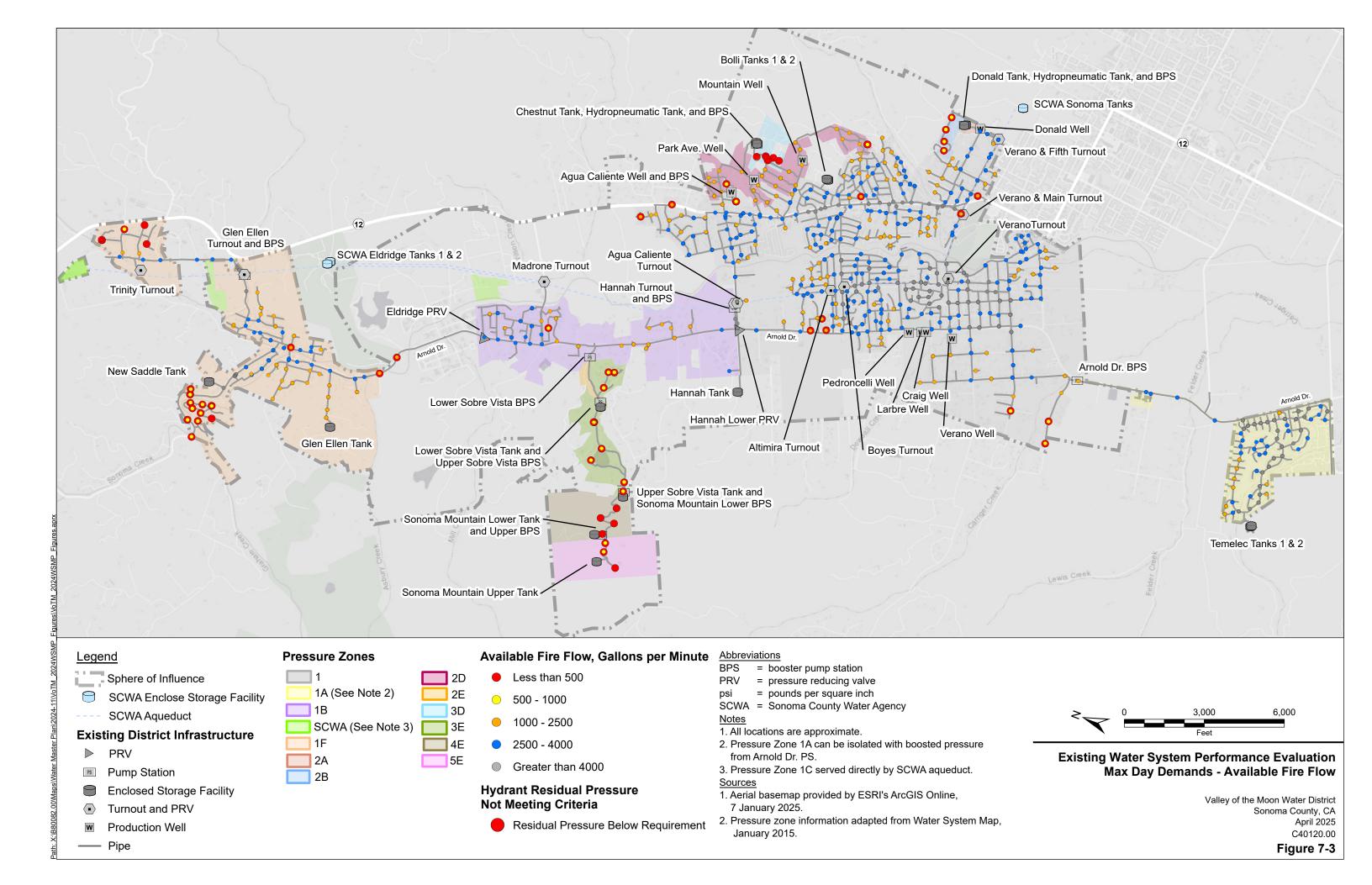
7.3.2 MDD Plus Fire Flow Results

Modeled fire flow availability is shown on Figure 7-3. Several hydrants cannot meet fire flow requirements, including:

- Multiple hydrants in the Trinity Oaks residential area in Pressure Zone 1F.
- Multiple hydrants in the residential areas along Warm Springs Road in Pressure Zone 1F.
- Three hydrants in the Glen Ellen along Arnold Drive in Pressure Zone 1F.
- One hydrant in Eldridge residential neighborhood in Pressure Zone 1B.
- Multiple hydrants with significant deficiencies throughout Sobre Vista and Sonoma Mountain in Pressure Zones 2E, 3E, 4E, and 5E.
- Multiple hydrants in commercial areas proximate to Highway 12 Pressure Zone 1.
- Three hydrants around the Altimira Middle School in Pressure Zone 1.
- Three hydrants in residential areas east of Arnold Drive in southern Pressure Zone 1.
- Two hydrants in residential neighborhoods of Pressure Zone 2D.
- Multiple hydrants in residential neighborhoods of Pressure Zone 3D.
- All hydrants in Pressure Zone 2B.







7.4 Future Scenario Evaluation

Modeled results for the Future scenario were evaluated to anticipate system deficiencies under future demand conditions. As discussed in Section 7.1, the Future scenario includes projects currently in planning or design phases, such as the Steel Replacement Project and the Altimira Fire Flow Improvement Project.

7.4.1 PHD Results

Model results for the Future scenario are similar to the Existing scenario, indicating that the planned development and increase in future demands do not significantly impact system pressures. Modeled system pressures at service connections under PHD ranged from approximately 24 psi to 136 psi, as shown on Figure 7-4. The lowest pressures experienced are located at similar locations as discussed in Section 7.3.1, and low-pressure areas have expanded slightly.

7.4.2 MDD Plus Fire Flow Results

Modeled fire flow availability is shown on Figure 7-5. Fire flow deficiencies in the Future scenario are generally similar to the Existing scenario, with exception of the three fire flow deficiencies near the Altimira School, which are modeled to be addressed by the Altimira Fire Flow Improvement Project.

7.5 Recommended Distribution System Capacity Improvements

Projects were developed to solve each hydraulic capacity deficiency identified in the hydraulic modeling evaluation. EKI first modeled the proposed facility improvement projects discussed in Section 5.4. For the remaining deficiencies, EKI identified pipeline projects to improve system pressures. In general, supply source and storage tank transmission pipelines that exhibited higher velocities and head losses in modeling results were targeted first, because upsizing these pipelines can significantly assist downstream pressure issues. Remaining fire flow deficiencies were addressed by upsizing distribution mains, adding new pipe connections, or by replacing the hydrant with a larger lateral. Lastly, transmission mains were identified for upsizing which could assist pressure deficiencies.

EKI modeled the remaining proposed improvements from 2019 under future demand conditions to confirm that the identified deficiencies had been addressed. Generally, the remaining projects identified in the 2019 WMP have been carried forward without any modifications, indicating that they are still appropriate solutions to the identified deficiencies. However, one project, the Agua Caliente Road Transmission Improvement, has been removed from the CIP list due to the proposed new pressure zone in the Agua Caliente Knolls area, as discussed below in Section 7.6, which also addresses the fire flow deficiencies identified in this area.

7.6 District Metered Areas

In addition to the recommended distribution system improvements above, EKI has incorporated three new pressure zone modifications identified by the District as new District Metered Areas (DMAs). The three DMAs are in (1) Glen Ellen in Pressure Zone 1F, (2) the Agua Caliente Knolls area in Pressure Zone 1, and (3) in the southern portion of Temelec in Zone 1A. The three new DMAs can be seen in Figure 7-6 as Pressure Zones 1G, 1H, and 1I, respectively. These new DMAs would be created by adding new PRV stations at the following locations:

- Pressure Zone 1G (Glen Ellen): New 6-inch PRV Station at the intersection of Arnold Drive and Carmel Avenue
- Pressure Zone 1H (Agua Caliente Knolls): New 8-inch PRV Station at the intersection of Kearney Avenue and East Agua Caliente Road, running parallel to the existing zone separating closed valve,



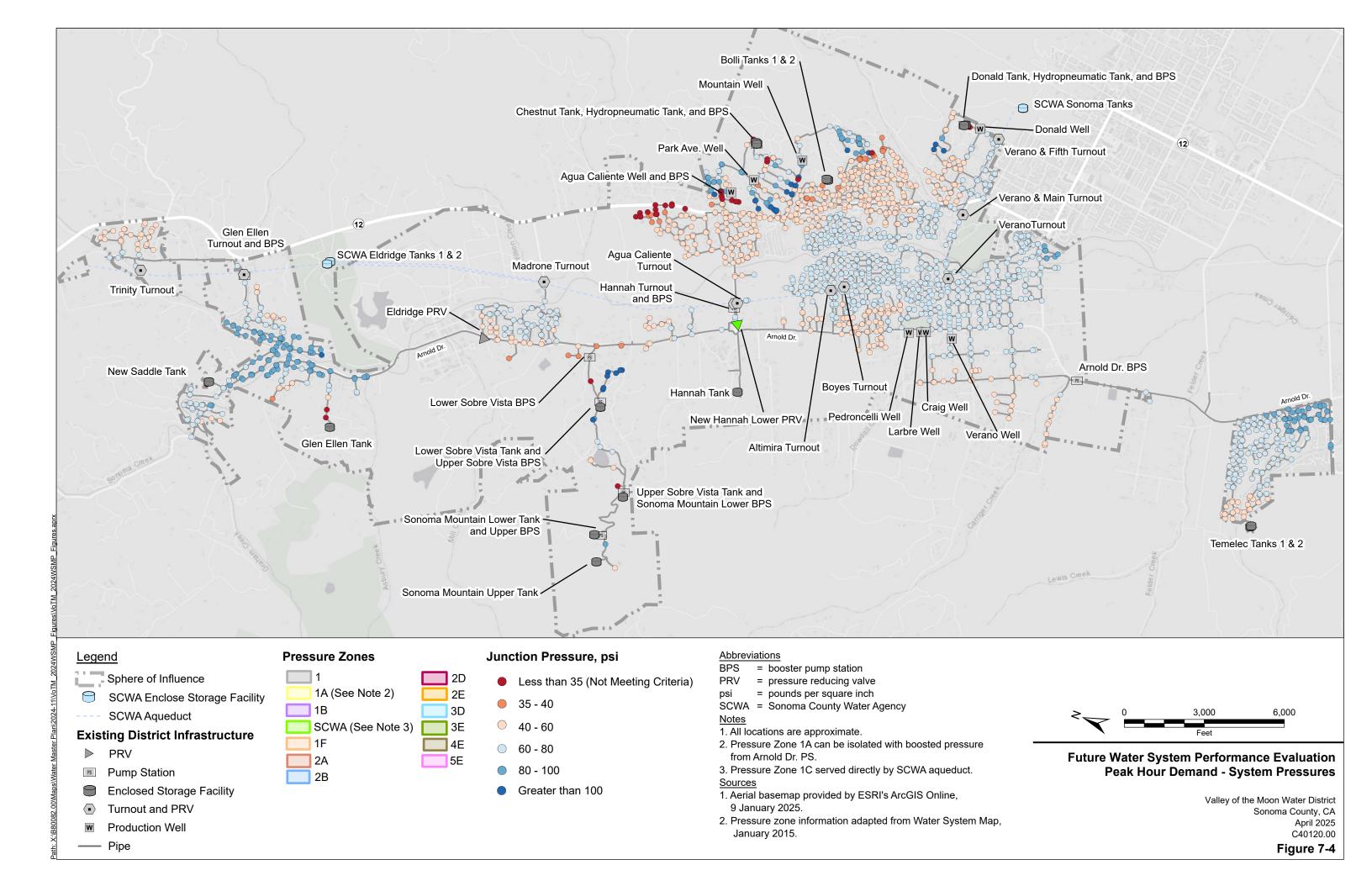
and new 12-inch PRV Stations (1) on West Agua Caliente Road east of the roundabout (2) on Highway 12 between Vailetti Drive and Sunnyside Avenue

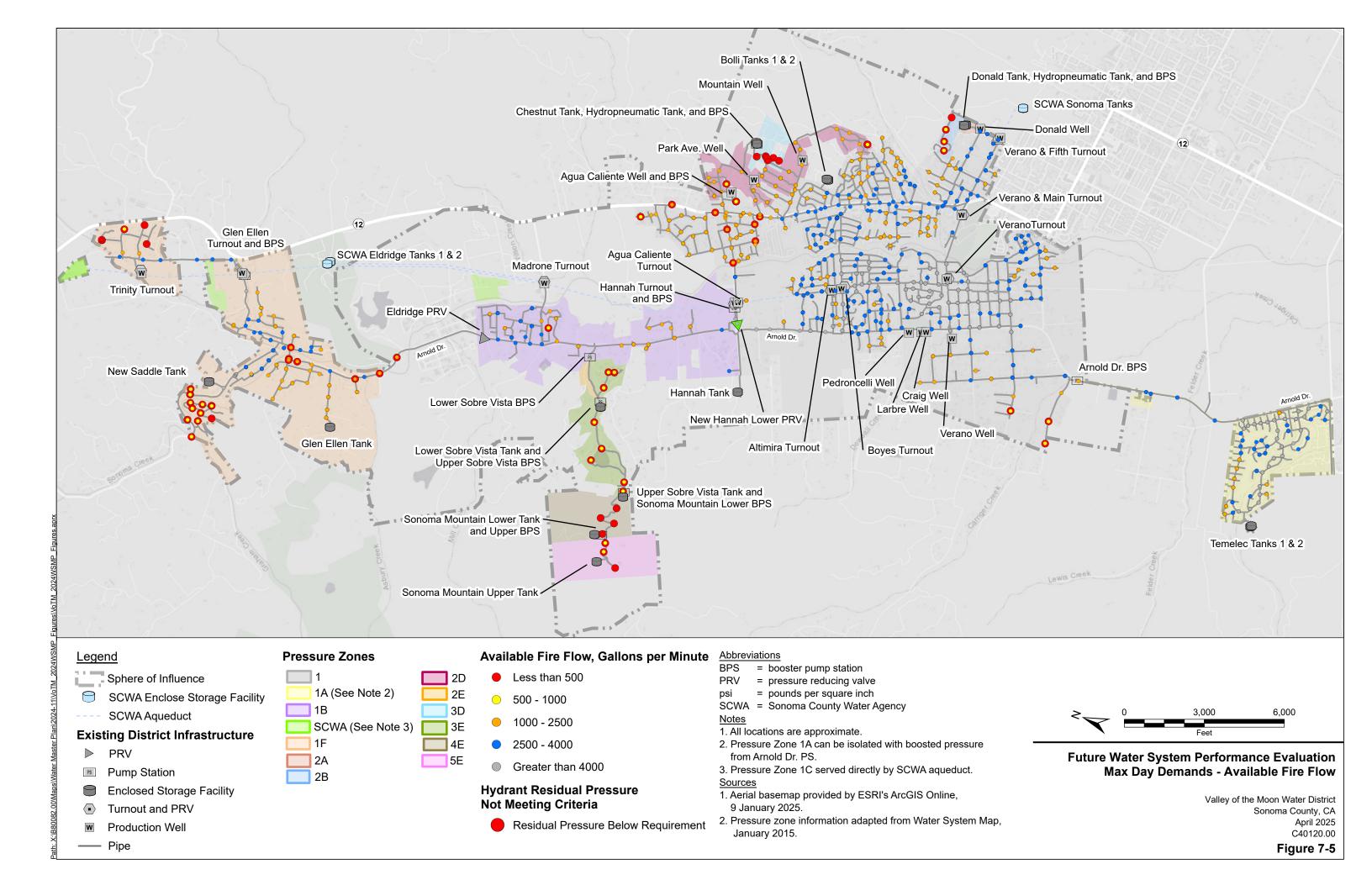
• Pressure Zone 1I (Temelec): New 6-inch PRV Stations (1) on Avenida Sebastiani between Via Colombard and Avenida Barbera and (2) on Arnold Drive between Mission Drive and Avenida Sebastiani, and close the valve on South Temelec Circle between Mission Drive and Herbazal Street.

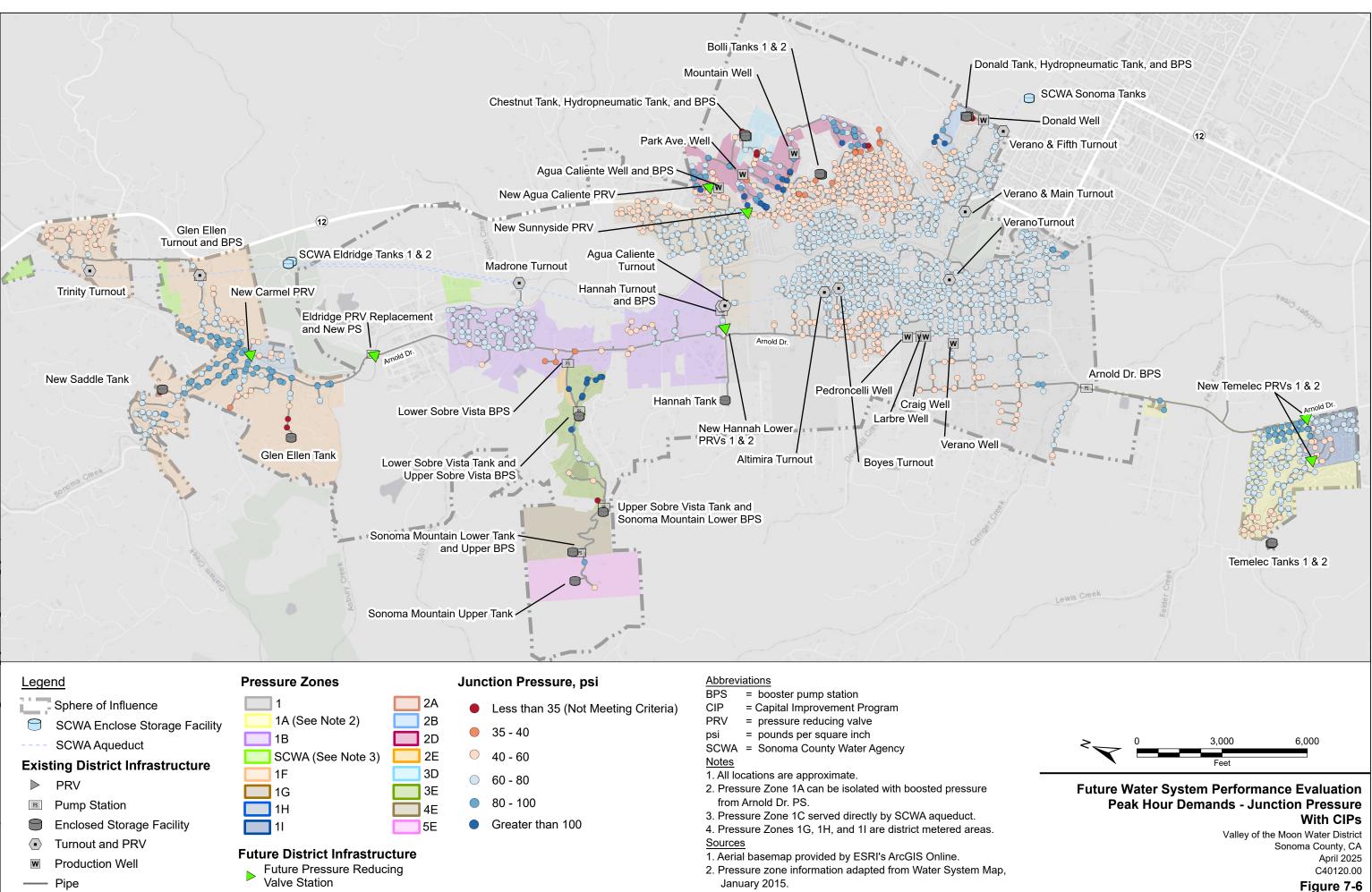
Modeled results for the Future scenario with the recommended water system capacity improvements and the new pressure zones were evaluated. As seen in Figure 7-6, the new DMAs reduce pressures from above 100 psi in the new Glen Ellen and Temelec DMAs to approximately 60 psi and increase pressure to above 35 psi in the Agua Caliente Knolls DMA to better meet system performance criteria. As seen in Figure 7-7, the new DMAs do not cause any new fire flow deficiencies and improve fire flow availability in the Agua Caliente Knolls Area.

These recommended hydraulic capacity projects are discussed in Section 8.

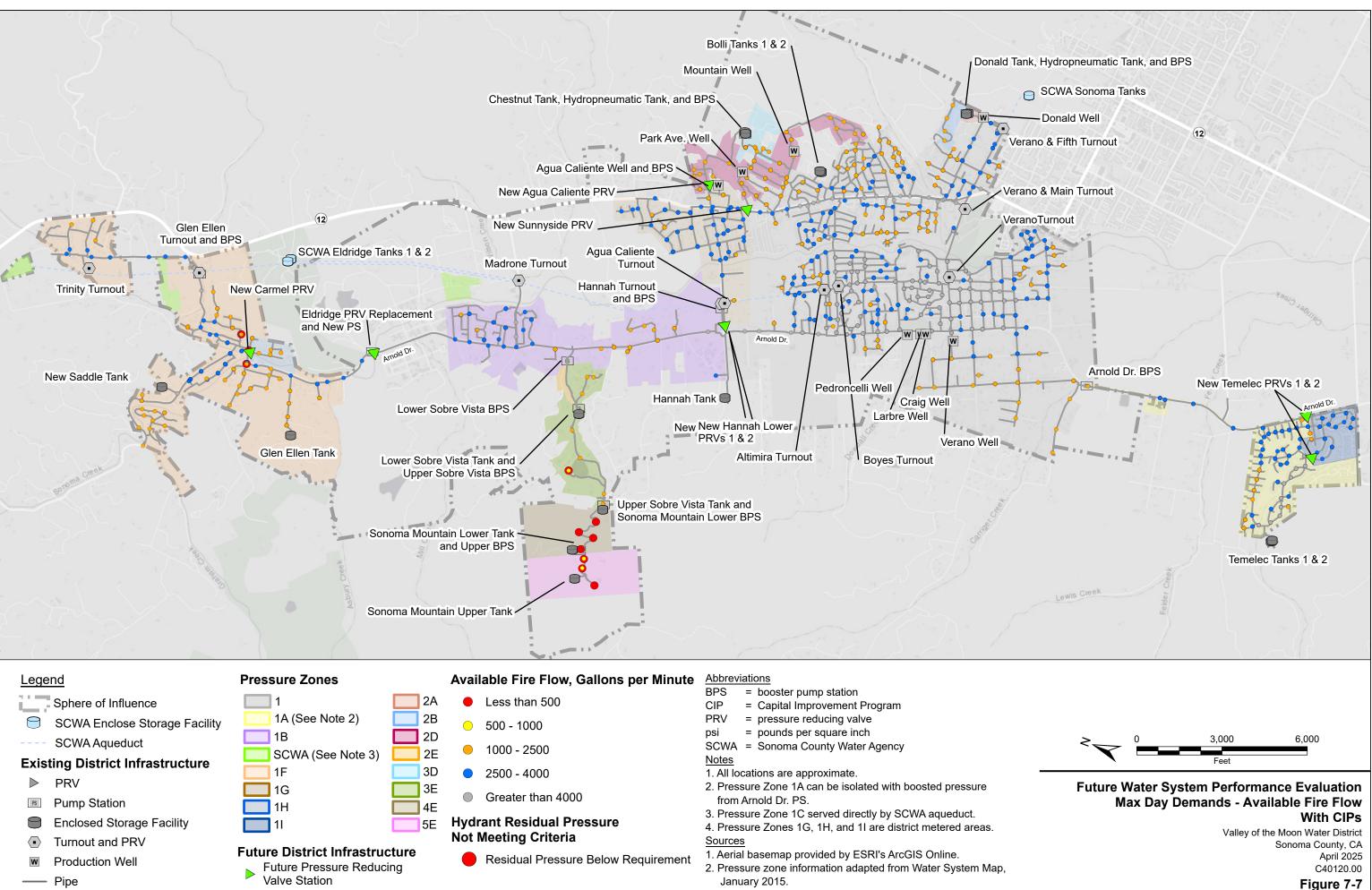








i uture	5 1103
Valve	Statio



8 RECOMMENDED CAPITAL IMPROVEMENT PROGRAM

This section summarizes the recommended update to the 2019 CIP that reprioritizes the remaining projects and includes additional newly identified projects to (1) rezone portions of the District as discussed in Section 7.6, and (2) additional pipeline condition projects to replace pipelines that have reached the end of their useful lives. EKI has developed opinions of probable cost (OPCs) and recommended priorities for each project.

8.1 CIP Costs

Costs for improvement projects have been estimated for both construction completed by District staff based on recent cost data provided by the District and by construction contractors based on recent bid results and EKI's experience with similar projects.

These costs are conceptual level estimates, considered to have an estimated accuracy range of -30% to +50%, suitable for use for budget forecasting, CIP development, and project evaluations, with the understanding that refinements to the project details and costs would be necessary as projects proceed to design and construction. An OPC for construction of each project has been developed using unit cost factors discussed below and are presented in December 2024 dollars based on an Engineering News Record (ENR) Construction Cost Index (CCI) of 15,400.54 (San Francisco).

The total project OPCs also include an additional 50-60% of the construction OPC to account for contingency, design, construction management, permitting, regulatory compliance, CEQA, and project implementation:

- Project Construction Contingency: 25% for pipeline replacements and 30% for all other projects
- Design: 10% for pipeline replacements and 15% for all other projects
- Construction Management: 5%
- Permitting, Regulatory Compliance, CEQA: 5%
- Project Implementation: 5%

8.1.1 Pipeline Project Costs

Unit costs for water pipeline projects are presented in Table 8-1. These costs vary by diameter both for installations by District staff and construction contractors. These cost factors assume open-trench construction and installation of C900 PVC pipe for all projects. The unit construction costs presented below generally include pipeline materials, trenching, placing and joining pipe, placing imported pipe bedding and backfill material, and partial asphalt pavement replacement, if required. These costs are representative of pipeline construction under normal conditions and would be higher for difficult cases.



Pipe Diameter	Estimated Cost (\$/linear foot)					
(inches)	Constructed by District Staff	Constructed by Contractor				
6	192	275				
8	230	332				
10	275	396				
12	332	473				

Table 8-1. Unit Construction Cost for Pipeline Projects

In addition to the unit construction cost for length of pipe installed, Table 8-2 presents additional unit costs that have been included to better estimate total project costs.

Table 8-2. Miscellaneous Costs for Pipeline Projects

ltore	Estimated Cost (\$/ea)					
ltem	Constructed by District Staff	Constructed by Contractor				
Hydrant Replacements	6,500	18,000				
Service Replacements	2,000	5,000				
Main Connections	2,500	10,000				

EKI assumes that additional fire hydrants, if requested by the Fire Department, would be funded by the Fire Department.

8.1.2 Treated Water Storage Tank Costs

Treated water storage tank costs are based on unit-volume cost factors which include the installation of above-grade steel storage tank, site piping, earthwork, paving, instrumentation, and all related sitework. Note that these costs are representative of construction conducted under normal excavation and foundation conditions and would be significantly higher for special or difficult foundation requirements.

8.1.3 Booster Pump Station Costs

The BPS OPCs conservatively assume full replacement the BPSs at the flow rates specified. This assumption has been made to capture the potential need for major BPS upgrades due to increased electrical loading or for condition-based replacements. Costs include installation of the booster pumps, pump station building, site piping, earthwork, paving, on-site backup/standby power generator, SCADA, and related sitework.

8.1.4 Groundwater Supply Well Costs

The new groundwater supply well OPC assumes the project will consistent of pilot hole drilling, e-logging, water quality/soil sampling, pilot hole reaming, well construction, well development, and installation of the necessary housing, pump, motor, electrical equipment, backup generator, SCADA equipment, discharge piping, and disinfection equipment. The construction OPC for a new 100 to 350 gpm well is estimated to be approximately \$1.0M to \$1.3M based on the costs for the recent installation of Well 5A, Park Well, and Chestnut Exploratory Well. The OPC assumes that no wellhead treatment is required besides chlorination.



8.1.5 Miscellaneous Costs

In addition to the costs presented above, other cost factors are presented in Table 8-3.

ltem	Estimated Cost (\$/ea)					
nem	District Staff	Contractor				
PRV Installation	90,000	120,000				
PRV Abandonment		15,000				
Tank Demolition		65,000				
BPS Removal		65,000				
Flow Meter	48,000	65,000				

Table 8-3. Miscellaneous Costs

PRV Station cost assumes a pre-assembled packaged PRV station that includes a 6-inch PRV and a 2-inch low-flow bypass PRV, a precast utility vault, and required connection piping, valves, and fittings.

8.1.6 Recommended Priorities

EKI has developed recommended priorities for each of the proposed improvements. Generally, EKI developed three levels of prioritization, described below:

- **Priority 1** Critical projects that should be initiated as soon as possible and completed over the next 5 years, including:
 - Projects to address significant fire flow deficiencies in sensitive areas (e.g., areas adjacent to urban-wildland boundary or a school);
 - Condition related projects identified by the District;
 - Projects to create new District Metered Areas to address pressure deficiencies or fire flow deficiencies;
- **Priority 2** Near-term projects that should be implemented in the next five to ten years, including:
 - Remaining projects that address significant fire flow deficiencies (i.e., greater than 40% deficient);
 - Projects to create new District Metered Areas to reduce excessive pressures;
 - Projects that address remaining storage and supply deficiencies; and
 - Projects that address minimum pressure deficiencies during normal operations.
- Priority 3 Long-term projects that should be implemented after priority two projects, including:
 - Projects that address the remaining fire flow deficiencies;
 - Replacement of aging 4-inch ACP; and
 - Other identified operational improvement.



8.2 Capital Improvement Projects

Figure 8-1 shows an overview of the recommended improvements P-2 through P-30. A summary of the recommended improvements, as well as the CIPs included in the District's existing 5-year CIP budget, are presented in Table 8-4. As shown in Table 8-4, the total OPC for the proposed CIP in December 2024 dollars is approximately \$27.0 million to \$34.9 million (depending on whether the pipeline projects will be constructed by the District or a construction contractor). It should be noted that the recommended CIP only identifies improvements at a master plan level and does not constitute a design of such improvements. Subsequent detailed design is required to determine the exact sizes and locations of these proposed improvements.

Project summary sheets are provided for each project in Appendix A. Each summary sheet includes a location map, a description of and justification for the proposed improvements, recommended priority, and estimated planning level OPC.



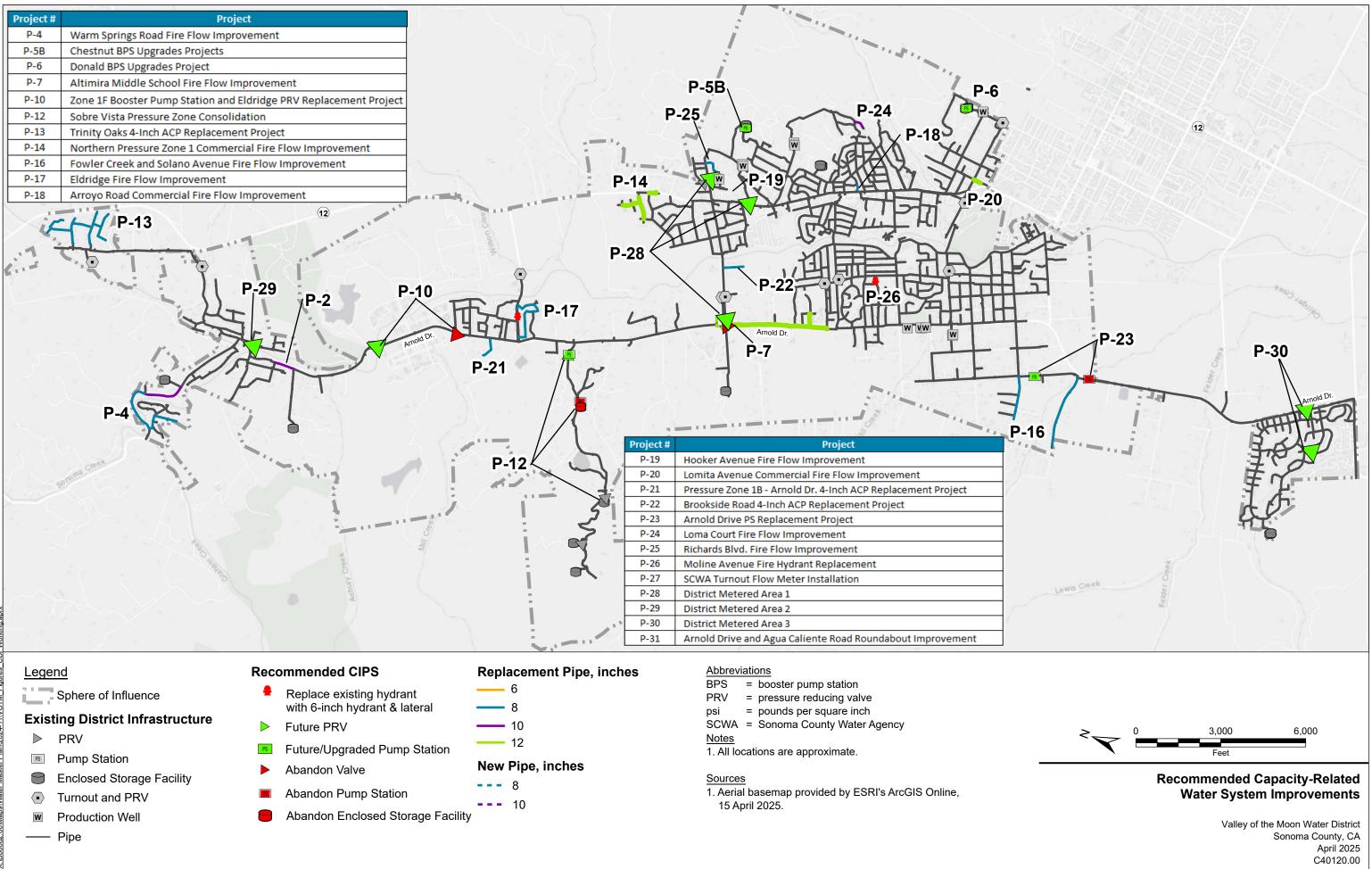


Figure 9-1

 Table 8-4

 Summary of Recommended Water System Capital Improvement Projects

				Recommended Pipe	Pipe Length	Total Pro (a)	oject OPC (b)
Project #	Project	Improvement Description	Priority	Diameter (in)	(Linear Feet)	District Staff	External Contractor
Facilities a	nd Maintenance Projects						
P-29	District Metered Area 2 (PZ-1G)	Install new 8-inch PRV station with flow metering at the intersection of Kearney Avenue and East Agua Caliente Road, running parallel to the existing zone separating closed valve, and new 12-inch PRV stations with flow metering (1) on West Agua Caliente Road east of the roundabout (2) on Highway 12 between Vailetti Drive and Sunnyside Avenue to create new pressure zone 1G in the Agua Caliente Knolls area.	1			\$670,000	\$920,000
P-27	SCWA Turnout Flow Meter Installation	Install flow meters at each of the SCWA turnout PRVs and integrate with SCADA system.	2			\$770,000	\$1,040,000
P-28	District Metered Area 1 (PZ-1H)	Install new 6-inch PRV station with flow metering at the corner of Arnold Drive and Carmel Ave and create new pressure zone in Glen Ellen.	2			\$230,000	\$330,000
P-30	District Metered Area 3 (PZ-11)	Install new 6-inch PRV stations with flow metering on (1) Avenida Sebastiani between Via Colombard and Avenida Barbera and (2) on Arnold Drive between Mission Drive and Avenida Sebastiani, and close the valve on South Temelec Circle between Mission Drive and Herbazal Street to create new pressure zone 1I in the Temelec Area.	2			\$450,000	\$620,000
Pipeline P	rojects			•		•	
D /	P-4 Warm Springs Road Fire Flow Improvement Replace existing 6-inch PVC, ACP, and DIP water mains with new 8-inch and 10-inch PVC water mains, replace 47 existing service connections, and replace four existing fire hydrants.		1	8	3,400	\$1,990,000	\$3,120,000
F-4			Ţ	10	1,500		
P-7	P-7 Altimira Middle School Fire Flow Improvement along Arnol	Replace existing 6-inch and 8-inch PVC and ACP water mains with new 12-inch PVC water mains along Arnold Drive, replace existing 6-inch pipe with new 8 and 12-inch pipe adjacent to Altimira	1	10	50	\$2,210,000	\$3,290,000
. ,		Middle School, replace 15 existing service connections, and replace three existing fire hydrants. This project could be combined with P-31 for efficiency.	rvice connections, and replace three existing fire hydrants. This 12 for efficiency.	12	4,235	<i>\$2,210,000</i>	<i></i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
P-13	Trinity Oaks 4-Inch ACP Replacement Project	Replace existing 4-inch ACP water mains with new 8-inch PVC water mains, replace 49 existing service connections, and replace six existing fire hydrants in the Trinity Oaks area. District to coordinate with Fire Department to determine if additional hydrants are needed. These hydrants would be funded by the Fire Department.	1	8	6,000	\$2,280,000	\$3,550,000
P-17	Eldridge Fire Flow Improvement	Replace existing 4-inch ACP water mains with new 8-inch PVC water mains, replace 49 existing service connections, and replace three existing fire hydrants in the Eldridge area. Abandon the 4-inch ACP water main on Madrone Avenue and reconnect services to existing 8-inch water main. This project has been identified as high priority due to the condition of the ACP water mains in this zone.	1	8	3,900	\$1,540,000	\$2,470,000
P-31	Arnold Drive and Agua Caliente Road Roundabout Improvement	Replace existing 8-inch ACP water mains with new 12-inch PVC water mains and relocate the existing Hannah Lower PRV out of the center of the new roundabout. This project has been identified as high priority due to the safety concerns with operating this PRV. This project could be combined with P-7 for efficiency.	1	12	2,000	\$1,230,000	\$1,760,000
P-14A	Northern Pressure Zone 1 Commercial Fire Flow Improvement - La Grama	Replace existing 6-inch water mains with new 12-inch PVC water mains, replace 3 existing service connections, and replace three existing fire hydrants.	2	12	1,425	\$760,000	\$1,150,000
P-16	Fowler Creek and Solano Avenue Fire Flow Improvement	Replace existing 6-inch ACP water mains with new 8-inch PVC water mains, replace ten existing service connections, and replace five existing fire hydrants.	2	8	4,200	\$1,550,000	\$2,380,000
P-14B	Northern Pressure Zone 1 Commercial Fire Flow Improvement - HWY 12	Replace existing 8-inch ACP water mains with new 12-inch PVC water mains, replace one existing service connections, and replace one existing fire hydrants. Based on discussions with the District, the commercial areas along HWY 12 have been vacant in this area for an extended period. This project is only recommended if new development occurs here.	3	12	280	\$160,000	\$260,000



 Table 8-4 (cont.)

 Summary of Recommended Water System Capital Improvement Projects

Project #	Project	Improvement Description	Priority	Recommended Pipe Diameter	Pipe Length	Total Pro (a)	oject OPC (b)
rroject #	i toject		Thorney	(in)	(Linear Feet)	District Staff	External Contractor
Pipeline P	rojects						
P-18	Arroyo Road Commercial Fire Flow Improvement	Install new 8-inch PVC water main between Highway 12 and Madera Road along Arroyo Road.	3	8	200	\$80,000	\$130,000
P-19	Hooker Avenue Fire Flow Improvement	Install new 8-inch PVC water main between Highway 12 and Hooker Ave.	3	8	550	\$200,000	\$300,000
P-20	Lomita Avenue Commercial Fire Flow Improvement	Replace existing 6-inch ACP water main with new 12-PVC water main along Lomita Avenue, replace two service connections, and replace one hydrant.	3	12	300	\$170,000	\$280,000
P-21	Pressure Zone 1B - Arnold Dr. 4-Inch ACP Replacement Project	Replace existing 4-inch ACP water main with new 8-inch PVC water main in Pressure Zone 1B west of Arnold Drive, and replace three existing service connections.	3	8	800	\$290,000	\$440,000
P-22	Brookside Road 4-Inch ACP Replacement Project	Replace existing 4-inch ACP water main with 8-inch PVC water main along Brookside Road and replace eight existing service connections.	3	8	800	\$300,000	\$470,000
P-24	Loma Court Fire Flow Improvement	Replace existing 6-inch with new 8-inch PVC along Loma Court, replace 11 existing service connections, and replace one existing fire hydrant.	3	8	500	\$220,000	\$370,000
P-25	Richards Blvd. Fire Flow Improvement	Replace existing 6-inch ACP and DIP water main with 8-inch PVC water main along Richards Blvd, replace four existing service connections, and one existing hydrant.	3	8	300	\$130,000	\$240,000
Pump Stat	ions, Tanks, and Wells					-	
P-5B	Chestnut BPS Upgrades Projects	Replace existing Chestnut BPS with two (2) 100-gpm domestic pumps and one (1) 1,000 gpm fire pump at 60 ft total dynamic head (TDH).	1				\$2,600,000
P-6	Donald BPS Upgrades Project	Replace existing Donald BPS with two (2) 115-gpm domestic pumps and one (1) 1,000 gpm fire pump at 220 ft TDH.	1				\$2,600,000
P-10	Zone 1F Booster Pump Station and Eldridge PRV Replacement Project	Install new PRV and BPS with a firm capacity of 450 gpm at 275 ft TDH. Abandon existing Eldridge PRV.	2				\$2,130,000
P-12	Sobre Vista Pressure Zone Consolidation	Replace Lower Sobre Vista BPS with a firm capacity of 290 gpm at 270 ft TDH; demolish Lower Sobre Vista Tank and Upper Sobre Vista BPS; connect PZ-2E and 3E; install individual service PRVs in former PZ-2E area; Replace existing 8-inch ACP water mains with new 8-inch PVC water mains.	2				\$2,650,000
P-23	Arnold Drive PS Replacement Project	Install new BPS with a firm capacity of 500 gpm along Orange Avenue. Demolish existing Arnold Drive BPS.	3				\$1,800,000
		TOTAL	WATER DIST	RIBUTION SYSTEM IMPR	OVEMENTS OPC (c)	\$27,010,000	\$34,900,000

Notes:

(a) Costs shown are presented in December 2024 dollars based on an ENR CCI of 15,400.54 (20-city average), with totals rounded to the nearest \$10,000.

(b) Costs for pipeline projects include construction contingency (25%), design (10%), construction management (5%), and Project Implementation (5%). Costs for other projects (i.e. BPS installations) include construction contingency (30%), design (15%), construction management (5%), permitting (5%), and Project Implementation (5%).

(c) Total district constructed OPC includes contractor costs for pump station, tanks, wells, and other projects not anticipated to be constructed by the District.

eki environment & water

9 **REFERENCES**

- SWCA, 2006. Restructured Agreement for Water Supply by and between Sonoma County Water Agency, City of Cotati, City of Petaluma, et al and Valley of the Moon Water District, 23 June 2006.
- EKI, 2019. Valley of the Moon Water District Water Master Plan, April 2019. EKI Environment & Water, Inc.
- EKI, 2021a. 2020 Urban Water Management Plan for Valley of the Moon Water District, June 2021. EKI Environment & Water, Inc.
- EKI, 2021b. Evaluation of Storage and Supply Requirements for Glen Ellen, 30 June 2021. EKI Environment & Water, Inc.
- EKI, 2022. Water Supply Assessment for the Sonoma Developmental Center Specific Plan, July 2022. EKI Environment & Water, Inc.
- EKI, 2023a. Hydraulic Evaluation for Sonoma Development Center, 25 September 2023. EKI Environment & Water, Inc.
- EKI, 2023b. Conceptual Transition Plan for Sonoma Development Center, 20 September 2023.
- EKI Environment & Water, Inc. EKI, 2024a. Water Supply Assessment for the 810 W Agua Caliente Development Project, March 2024. EKI Environment & Water, Inc.
- EKI, 2024b. Chestnut Exploratory Borehole Data Review Chestnut Water Storage Tank Site, 9 April 2024. EKI Environment & Water, Inc.



Appendix A

CIP Project Detail Sheets





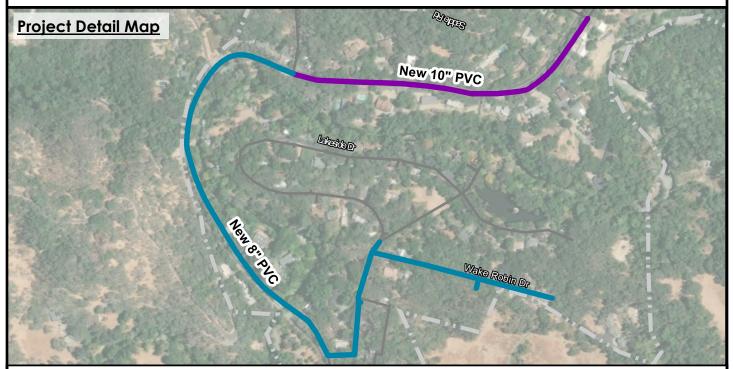
Project ID: P-4 Project Priority Level: 1

Description: Warm Springs Road Fire Flow Improvement

Location: Warm Springs Rd, Lakeside Rd, and Wake Robin Dr.

Improvement Details: Replace existing 6-inch PVC, ACP, and DIP water mains with new 8-inch and 10-inch PVC water mains, replace 47 existing service connections, and replace four existing fire hydrants.

Justification: Addresses significant fire flow deficiencies in residential areas near the wildland-urban interface.



Increase the second sec	Recommended	ended Quantity		Construction by District		Construction by Contracto	
Improvement Type	Diameter	Quan	uty	Unit Cost	Total Cost	Cost Factor	Total Cost
Warm Springs Road Fire Flow Improvemen	Warm Springs Road Fire Flow Improvement						
Replacement Pipeline	8	3,400	LF	\$230	\$782,300	\$332	\$1,128,800
Replacement Pipeline	10	1,500	LF	\$275	\$412,200	\$396	\$594,400
Hydrant Replacement		4	EA	\$6,500	\$26,000	\$18,000	\$72,000
Service Connection Replacement		47	EA	\$2,000	\$94,000	\$5,000	\$235,000
Main Tie-ins		5	EA	\$2,500	\$12,500	\$10,000	\$50,000
	Constr	uction (Conti	ngency (25%)	\$331,800		\$520,100
Construction OPC					\$1,658,800		\$2,600,300
Engineering, Administration, and Permitting Costs (25%)					\$331,800		\$520,100
Total OPC S					\$1,990,000		\$3,120,000



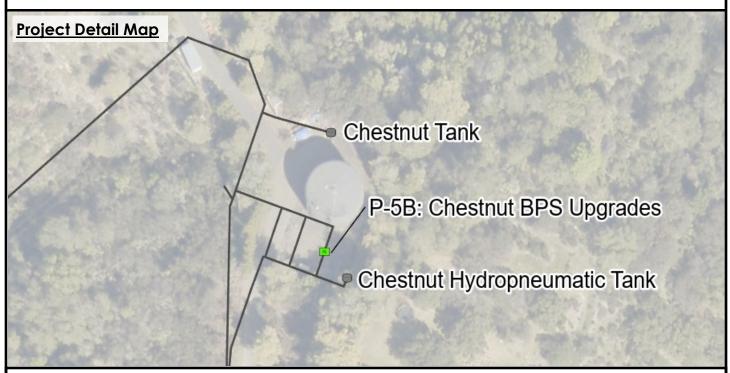
Project ID: P-5B Project Priority Level: 1

Description: Chestnut BPS Upgrades Projects

Location: Pressure Zone 3D BPS

Improvement Details: Replace existing Chestnut BPS with two (2) 100-gpm domestic pumps and one (1) 1,000 gpm fire pump at 60 ft total dynamic head (TDH).

Justification: Addresses supply deficiency and significant fire flow deficiencies in residential areas near the wildlandurban interface and replaces aging pump station facility.



Improvement Type	Recommended	Quantity	Constructio	n by District	Construction by Contracto			
Improvement Type	Diameter		Unit Cost	Total Cost	Cost Factor	Total Cost		
Chestnut BPS Upgrades Projects								
BPS Improvement - Fire Pump Installation		1 LS			\$1,625,900	\$1,625,900		
	Construction Contingency (30%)					\$487,800		
		Cons	truction OPC			\$2,113,700		
Engineering, Administration, and Permitting Costs (30%)						\$487,800		
Total OPC						\$2,600,000		



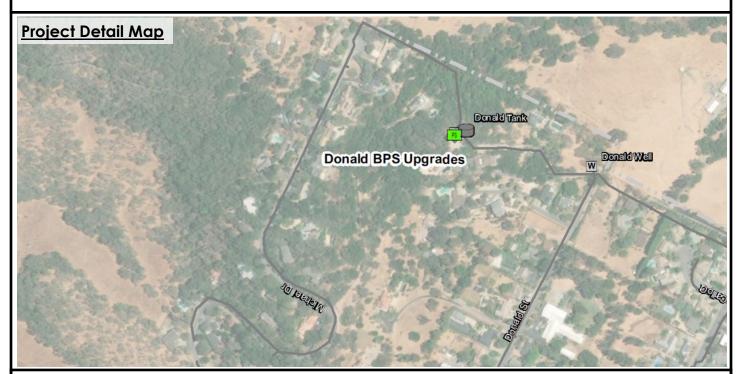
Project ID: P-6 Project Priority Level: 1

Description: Donald BPS Upgrades Project

Location: Pressure Zone 2B BPS

Improvement Details: Replace existing Donald BPS with two (2) 115-gpm domestic pumps and one (1) 1,000 gpm fire pump at 220 ft TDH.

<u>Justification</u>: Addresses supply deficiency and significant fire flow deficiencies in residential areas near the wildlandurban interface and replaces aging pump station facility.



Improvement Type	Improvement Type Recommended Quantity Construction		Constructio	n by District	Construction by Contractor	
improvement type			Unit Cost	Total Cost	Cost Factor	Total Cost
Donald BPS Upgrades Project						
BPS Improvement - Fire Pump Replace- ment		1 LS			\$1,625,900	\$1,625,900
	Constru	uction Conti	ngency (30%)			\$487,800
	Construction OPC					\$2,113,700
Engineering, Administration, and Permitting Costs (30%)					\$487,800	
	Total OPC					\$2,600,000



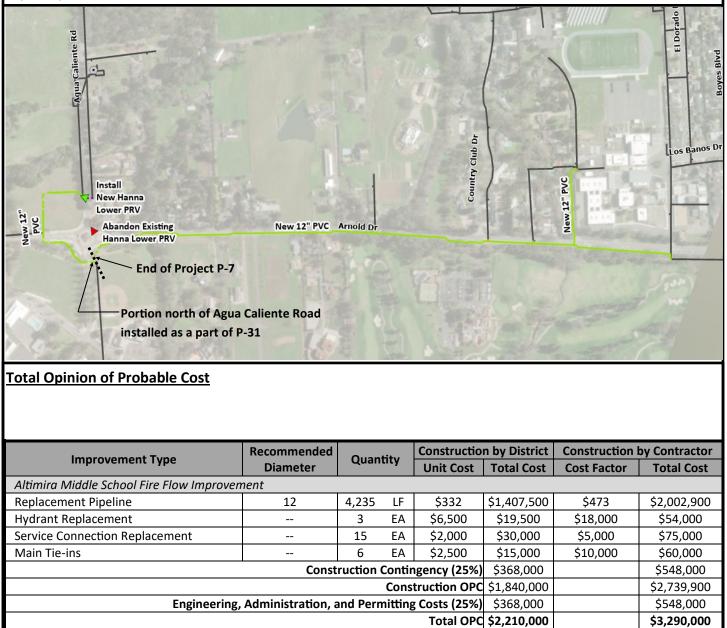
Project ID: P-7 Project Priority Level: 1

Description: Altimira Middle School Fire Flow Improvement

Location: Arnold Dr between Agua Caliente Rd and Boyes Blvd.

<u>Improvement Details</u>: Replace existing 6-inch and 8-inch PVC and ACP water mains with new 10-inch and 12-inch PVC water mains along Arnold Drive, replace existing 6-inch pipe with new 12-inch pipe adjacent to Altimira Middle School, replace 15 existing service connections, and replace three existing fire hydrants. This project could be combined with project P-31 for efficiency.

Justification: Addresses significant fire flow deficiency near Altimira Middle School and increases PZ-1 transmission capability.





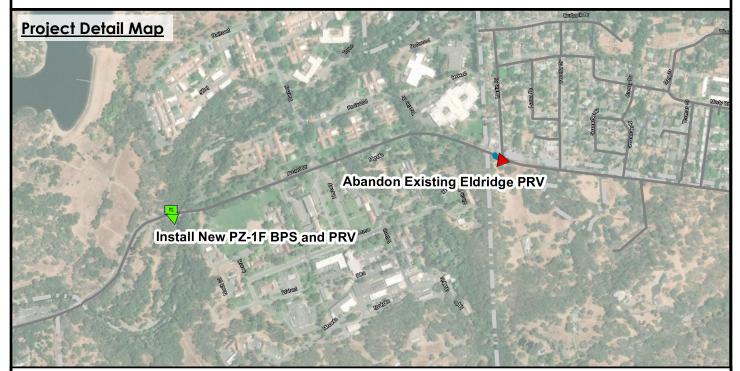
Project ID: P-10 Project Priority Level: 2

Description: Zone 1F Booster Pump Station and Eldridge PRV Replacement Project

Location: Near 14500 Arnold Dr.

Improvement Details: Install new PRV and BPS with a firm capacity of 450 gpm at 275 ft TDH. Abandon existing Eldridge PRV.

Justification: Addresses supply deficiency and fire flow deficiencies in PZ-1F and improves system redundancy.



	Recommended	Quantity		Constructio	n by District	Construction	by Contractor
Improvement Type	Diameter	Qua	nuty	Unit Cost	Total Cost	Cost Factor	Total Cost
Zone 1F Booster Pump Station and Eldridge I							
BPS Improvement		1	LS			\$1,262,900	\$1,262,900
PRV Installation		1	EA			\$120,000	\$120,000
Main Tie-ins		2	EA			\$10,000	\$20,000
Abandonment of Existing PRV		1	EA			\$15,000	\$15,000
	Constru	uction	Conti	ngency (25%)			\$354,500
Construction OPC						\$1,772,400	
Engineering, Administration, and Permitting Costs (25%)						\$354,500	
Total OPC						\$2,130,000	



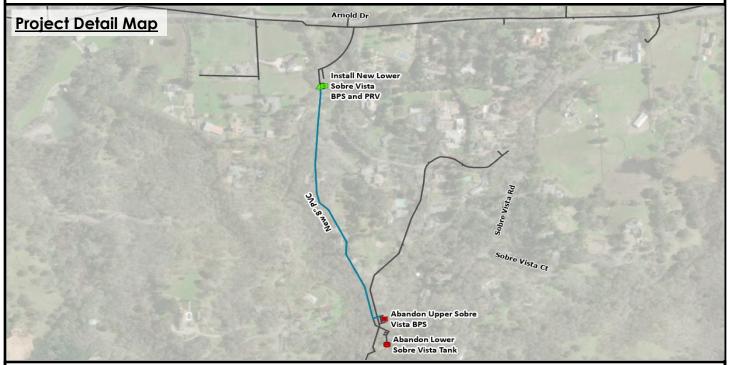
Project ID: P-12 Project Priority Level: 2

Description: Sobre Vista Pressure Zone Consolidation

Location: Lower Sobre Vista BPS and Tank

Improvement Details: Replace Lower Sobre Vista BPS with a firm capacity of 265 gpm at 270 ft TDH; demolish Lower Sobre Vista Tank and Upper Sobre Vista BPS; connect PZ-2E and 3E; replace existing 8-inch ACP pipes with 8-inch PVC pipe; install individual service PRVs in former PZ-2E area; install new PRV station between Pressure Zones 2E/3E and 1B.

<u>Justification</u>: Addresses supply and storage deficiencies in PZ-2E and PZ-3E, decommissions aging Lower Sobre Vista Tank, and improves system operations and redundancy.



	Recommended	0		Constructio	n by District	Construction	by Contractor
Improvement Type	Diameter	Quan	uty	Unit Cost	Total Cost	Cost Factor	Total Cost
Sobre Vista Pressure Zone Consolidation							
Replacement BPS - 265 gpm		1	LS			\$1,041,300	\$1,041,300
Tank Removal		1	LS			\$65,000	\$65,000
Replacement Pipeline	8	1,260	LF	\$230	\$289,900	\$332	\$418,300
PRV Installation		1	EA			\$120,000	\$120,000
Main Tie-ins		4	EA			\$2,500	\$10,000
	Const	ruction	Contir	ngency (30%)			\$496,400
			Cons	truction OPC			\$2,151,000
Engineering,	Engineering, Administration, and Permitting Costs (30%)						\$496,400
	Total OPC						\$2,650,000



Project ID: P-13 Project Priority Level: 2

Description: Trinity Oaks 4-Inch ACP Replacement Project

Location: Bonnie Way, Sylvia Dr, Adine Ct.

Improvement Details: Replace existing 4-inch ACP water mains with new 8-inch PVC water mains, replace existing service connections, and replace existing fire hydrants in the Trinity Oaks area. District to coordinate with Fire Department to determine if additional hydrants are needed. These hydrants would be funded by the Fire Department.

Justification: Addresses fire flow deficiencies in the area and replaces aging 4-inch ACP.



Improvement Type	Recommended	0		Constructio	n by District	Construction	by Contractor
Improvement Type	Diameter	Quantity		Unit Cost	Total Cost	Cost Factor	Total Cost
Trinity Oaks 4-Inch ACP Replacement Proje	ct						
Replacement Pipeline	8	6,000	LF	\$230	\$1,380,500	\$332	\$1,992,000
Hydrant Replacement		6	EA	\$6,500	\$39,000	\$18,000	\$108,000
Service Connection Replacement		49	EA	\$2,000	\$98,000	\$5,000	\$245,000
Main Tie-ins		2	EA	\$2,500	\$5,000	\$10,000	\$20,000
	Constr	uction (Conti	ngency (25%)	\$380,600		\$591,300
			Cons	truction OPC	\$1,903,100		\$2,956,300
Engineering, Administration, and Permitting Costs (25%)							\$591,300
	Total OPC						\$3,550,000



Project ID: P-14A Project Priority Level: 2

Description: Northern Pressure Zone 1 Commercial Fire Flow Improvement

Location: La Grama Dr

Improvement Details: Replace existing 6-inch ACP water mains with new 12-inch PVC water mains, replace 2 existing service connections, and replace one existing fire hydrants.

Justification: Addresses significant commercial fire flow deficiencies on a dead-end water main.



Improvement Turpe	Recommended	Quan	Li t.,	Constructio	n by District	Construction	by Contractor
Improvement Type	Diameter	Quantity		Unit Cost	Total Cost	Cost Factor	Total Cost
Northern Pressure Zone 1 Commercial Fire							
Replacement Pipeline	12	1,425	LF	\$332	\$473,600	\$473	\$674,000
Hydrant Replacement		3	EA	\$6,500	\$19,500	\$18,000	\$54,000
Service Connection Replacement		2	EA	\$2,000	\$4,000	\$5,000	\$10,000
Main Tie-ins		3	EA	\$2,500	\$7,500	\$10,000	\$30,000
	Const	ruction (Contir	ngency (25%)	\$126,200		\$192,000
			Cons	truction OPC	\$630,800		\$960,000
Engineering, Administration, and Permitting Costs (25%)							\$192,000
				Total OPC	\$760,000		\$1,150,000



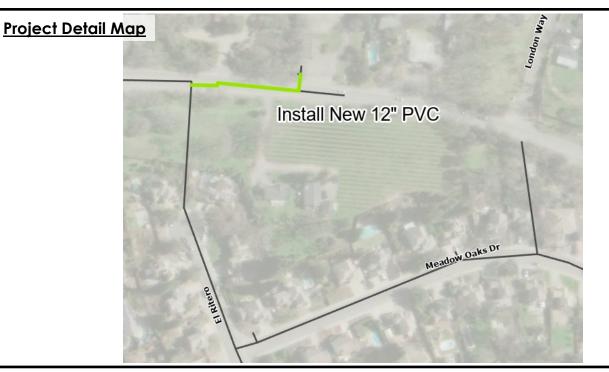
Project ID: P-14B Project Priority Level: 3

Description: Northern Pressure Zone 1 Commercial Fire Flow Improvement

Location: Highway 12

<u>Improvement Details</u>: Replace existing 8-inch ACP water mains with new 12-inch PVC water mains, replace one existing service connections, and replace one existing fire hydrant. Based on discussions with the District, the commercial businesses along HWY 12 have been vacant in this area and the southeastern portion of this project could be removed from this Project.

Justification: Addresses significant commercial fire flow deficiencies.



	Recommended	0	 .	Constructio	n by District	Construction	by Contractor
Improvement Type	Diameter	Quantity		Unit Cost	Total Cost	Cost Factor	Total Cost
Northern Pressure Zone 1 Commercial Fire	Flow Improveme	nt - HW	Y 12				
Replacement Pipeline	12	280	LF	\$332	\$93,100	\$473	\$132,400
Hydrant Replacement		1	EA	\$6,500	\$6,500	\$18,000	\$18,000
Service Connection Replacement		1	EA	\$2,000	\$2,000	\$5,000	\$5,000
Main Tie-ins		2	EA	\$2,500	\$5,000	\$10,000	\$20,000
	Const	ruction	Contir	igency (25%)	\$26,700		\$43,900
			Cons	truction OPC	\$133,300		\$219,300
Engineering,	Administration, a	nd Perr	nitting	; Costs (25%)	\$26,700		\$43,900
				Total OPC	\$160,000		\$260,000



Project ID: P-16 Project Priority Level: 2

Description: Fowler Creek and Solano Avenue Fire Flow Improvement

Location: Fowler Creek Rd and Solano Ave.

Improvement Details: Replace existing 6-inch ACP water mains with new 8-inch PVC water mains, replace ten existing service connections, and replace five existing fire hydrants.

Justification: Addresses significant fire flow deficiencies on dead-end residential streets.



Improvement Type	Recommended	0	÷+.,	Construction by District		Construction	by Contractor
Improvement Type	Diameter	Quantity		Unit Cost	Total Cost	Cost Factor	Total Cost
Fowler Creek and Solano Avenue Fire Flow	Improvement						
Replacement Pipeline	8	4,200	LF	\$230	\$966,400	\$332	\$1,394,400
Hydrant Replacement		5	EA	\$6,500	\$32,500	\$18,000	\$90,000
Service Connection Replacement		10	EA	\$2,000	\$20,000	\$5,000	\$50,000
Main Tie-ins		5	EA	\$2,500	\$12,500	\$10,000	\$50,000
	Constr	uction (Conti	ngency (25%)	\$257,900		\$396,100
			Cons	truction OPC	\$1,289,300		\$1,980,500
Engineering, Administration, and Permitting Costs (25%)					\$257,900		\$396,100
					\$1,550,000		\$2,380,000



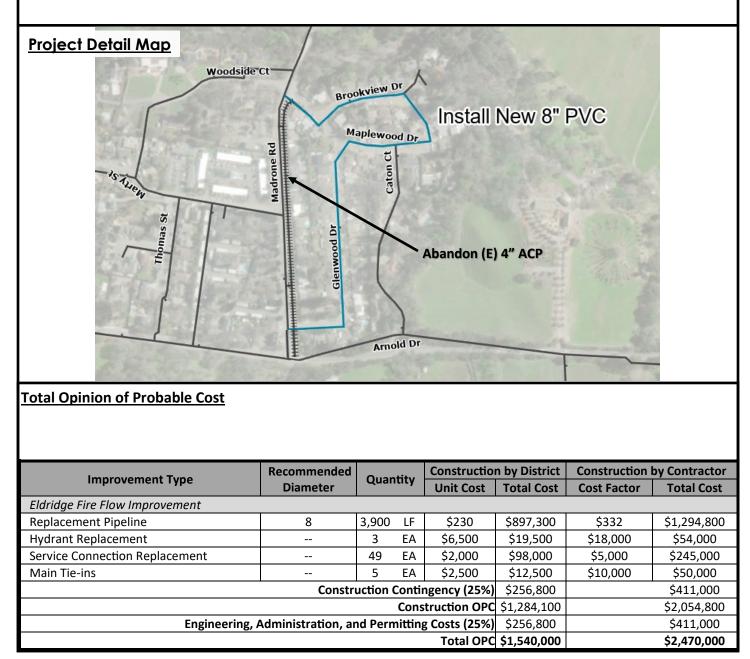
Project ID: P-17 Project Priority Level: 1

Description: Eldridge Fire Flow Improvement

Location: Madrone Rd, Glenwood Dr, Maplewood Dr, and Oakwood Dr.

<u>Improvement Details</u>: Replace existing 4-inch ACP water mains with new 8-inch PVC water mains, replace 49 existing service connections, and replace three existing fire hydrants in the Eldridge area. Abandon the 4-inch ACP water main on Madrone Avenue and reconnect services to existing 8-inch water main. This project has been upgraded to level 1 priority since the 2019 WMP due to the deteriorating condition of the ACP water mains in this zone.

Justification: Addresses fire flow deficiencies in the residential area and replaces aging 4-inch ACP.





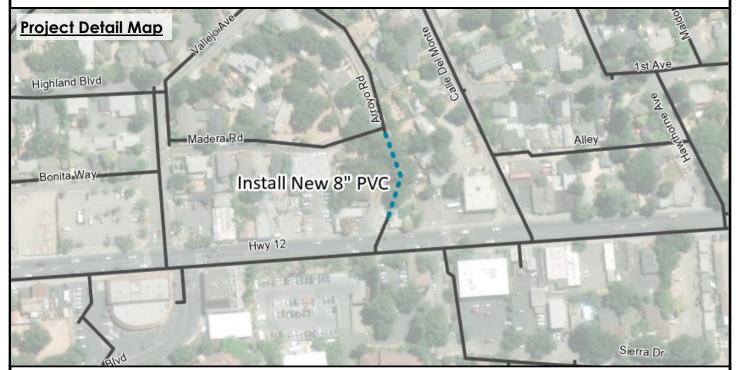
Project ID: P-18 Project Priority Level: 3

Description: Arroyo Road Commercial Fire Flow Improvement

Location: Arroyo Rd.

Improvement Details: Install new 8-inch PVC water main between Highway 12 and Madera Road along Arroyo Road.

Justification: Addresses commercial fire flow deficiencies.



Improvement Type	Recommended	Quantity		Construction by District		Construction by Contractor	
Improvement Type	Diameter	Quan	uty	Unit Cost	Total Cost	Cost Factor	Total Cost
Arroyo Road Commercial Fire Flow Improve	ement						
New Pipeline Installation	8	200	LF	\$230	\$46,000	\$332	\$66,400
Main Tie-ins		2	EA	\$2,500	\$5,000	\$10,000	\$20,000
	Con	structio	n Con	tingency (25%)	\$12,800		\$21,600
			Со	nstruction OPC	\$63,800		\$108,000
Engineerin	Engineering, Administration, and Permitting Costs (25%) \$12,800						
				Total OPC	\$80,000		\$130,000



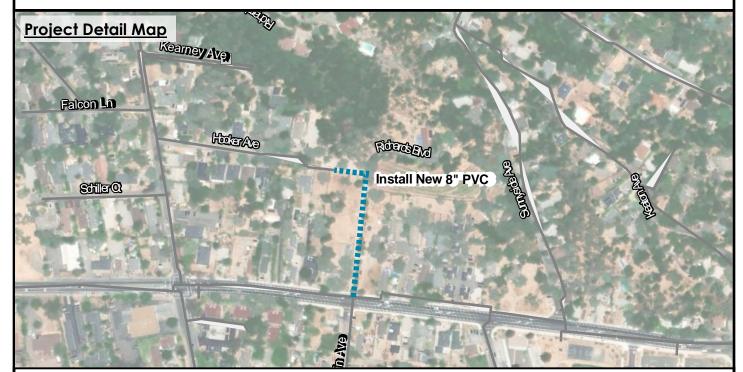
Project ID: P-19 Project Priority Level: 3

Description: Hooker Avenue Fire Flow Improvement

Location: Between Highway 12 and Hooker Ave.

Improvement Details: Install new 8-inch PVC water main between Highway 12 and Hooker Ave.

Justification: Addresses residential fire flow deficiencies.



Improvement Type	Recommended	Quantity	Construction by District		Construction by Contracto	
Improvement Type	Diameter	Quantity	Unit Cost	Total Cost	Cost Factor	Total Cost
Hooker Avenue Fire Flow Improvement						
New Pipeline Installation	8	550 LF	\$230	\$126,500	\$332	\$182,600
Main Tie-ins		2 EA	\$2,500	\$5 <i>,</i> 000	\$10,000	\$20,000
	Constr	uction Conti	ngency (25%)	\$32,900		\$50,700
		Cons	struction OPC	\$164,400		\$253,300
Engineering, Administration, and Permitting Costs (25%)						\$50,700
Total OPC						\$300,000



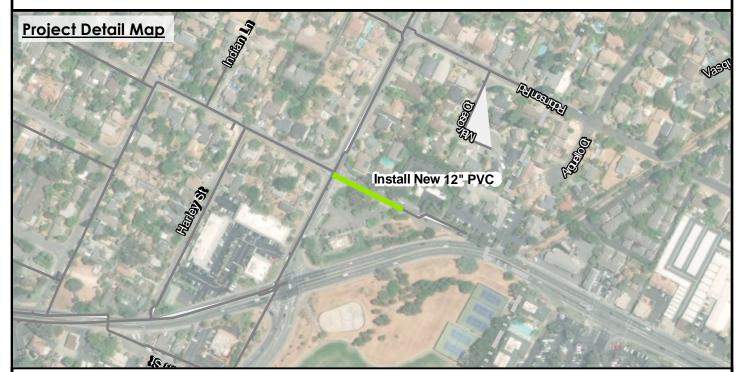
Project ID: P-20 Project Priority Level: 3

Description: Lomita Avenue Commercial Fire Flow Improvement

Location: Lomita Ave.

Improvement Details: Replace existing 6-inch ACP water main with new 12-PVC water main, replace two service connections, and replace one hydrant.

Justification: Addresses commercial fire flow deficiencies.



Improvement Type	Recommended	Quar		Construction by District		Construction	by Contractor
Improvement Type	Diameter		illy	Unit Cost	Total Cost	Cost Factor	Total Cost
Lomita Avenue Commercial Fire Flow Impre	ovement						
Replacement Pipeline	12	300	LF	\$332	\$99,700	\$473	\$141,900
Hydrant Replacement		1	EA	\$6,500	\$6,500	\$18,000	\$18,000
Service Connection Replacement		2	EA	\$2,000	\$4,000	\$5,000	\$10,000
Main Tie-ins		2	EA	\$2,500	\$5 <i>,</i> 000	\$10,000	\$20,000
	Constru	uction	Conti	ngency (25%)	\$28,800		\$47,500
			Cons	truction OPC	\$144,000		\$237,400
Engineering, Administration, and Permitting Costs (25%)					\$28,800		\$47,500
	Total OPC						\$280,000



Project ID: P-21 Project Priority Level: 3

Description: Pressure Zone 1B - Arnold Drive 4-Inch ACP Replacement Project

Location: Private road near 15263 Arnold Dr.

Improvement Details: Replace existing 4-inch ACP water main with new 8-inch PVC water main in Pressure Zone 1B west of Arnold Drive and replace three existing service connections.

Justification: Replaces aging 4-inch ACP.



	Recommended	Quar		Construction by District		Construction by Contractor		
Improvement Type	Diameter	Quar	itity	Unit Cost	Total Cost	Cost Factor	Total Cost	
Pressure Zone 1B - Arnold Dr. 4-Inch ACP R	Pressure Zone 1B - Arnold Dr. 4-Inch ACP Replacement Project							
Replacement Pipeline	8	800	LF	\$230	\$184,100	\$332	\$265,600	
Service Connection Replacement		3	EA	\$2,000	\$6,000	\$5,000	\$15,000	
Main Tie-ins		1	EA	\$2,500	\$2,500	\$10,000	\$10,000	
	Constru	uction	Contii	ngency (25%)	\$48,200		\$72,700	
			Cons	truction OPC	\$240,800		\$363,300	
Engineering, Administration, and Permitting Costs (25%)							\$72,700	
	Total OPC						\$440,000	



Project ID: P-22 Project Priority Level: 3

Description: Brookside Road 4-Inch ACP Replacement Project

Location: Brookside Rd.

Improvement Details: Replace existing 4-inch ACP water main with 8-inch PVC water main along Brookside Road , and replace eight existing service connections.

Justification: Replaces aging 4-inch ACP.



Improvement Type	Recommended	Quan	+i+.,	Construction by District		Construction	by Contractor
Improvement Type	Diameter	Quan	uty	Unit Cost	Total Cost	Cost Factor	Total Cost
Brookside Road 4-Inch ACP Replacement F	Project						
Replacement Pipeline	8	800	LF	\$230	\$184,100	\$332	\$265,600
Service Connection Replacement		8	EA	\$2,000	\$16,000	\$5,000	\$40,000
Main Tie-ins		1 EA		\$2,500	\$2,500	\$10,000	\$10,000
	Con	structio	n Con	tingency (25%)	\$50,700		\$78,900
			Со	nstruction OPC	\$253,300		\$394,500
Engineering, Administration, and Permitting Costs (25%)							\$78,900
				Total OPC	\$300,000		\$470,000



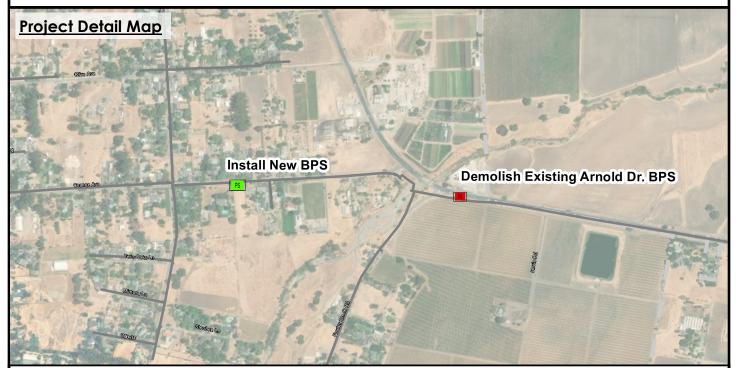
Project ID: P-23 Project Priority Level: 3

Description: Arnold Drive PS Replacement Project

Location: Near 19328 Orange Ave.

Improvement Details: Install new BPS with a firm capacity of 500 gpm along Orange Avenue. Demolish existing Arnold Drive BPS.

<u>Justification</u>: Replaces aging infrastructure and improves operations and maintenance for a facility needed for operational flexibility.



Improvement Type	Recommended	0	atitu/	Construction by District		Construction	by Contractor
Improvement Type	Diameter	Quantity		Unit Cost	Total Cost	Cost Factor	Total Cost
Arnold Drive PS Replacement Project							
Replacement BPS - 500 gpm		1	LS			\$1,312,400	\$1,041,300
Decommission PS		1	EA			\$65,000	\$65,000
Main Tie-ins		2	EA			\$10,000	\$20,000
	Constru	uction	Conti	ngency (30%)			\$337,900
			Cons	truction OPC			\$1,464,200
Engineering, Administration, and Permitting Costs (30%)							\$337,900
				Total OPC			\$1,800,000



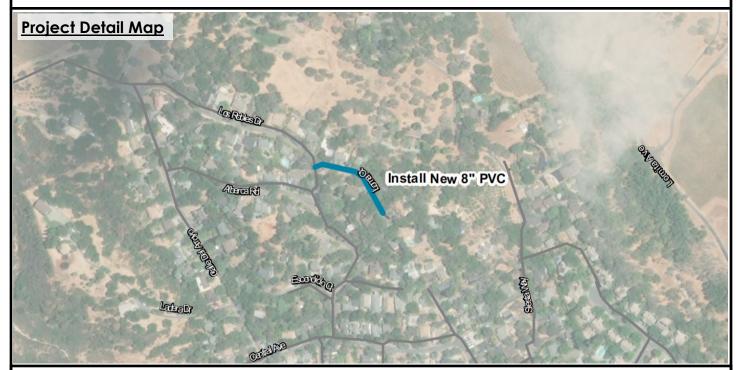
Project ID: P-24 Project Priority Level: 3

Description: Loma Court Fire Flow Improvement

Location: Loma Ct.

Improvement Details: Replace existing 6-inch with new 8-inch PVC along Loma Court, replace 11 existing service connections, and replace one existing fire hydrant.

Justification: Addresses minor fire flow deficiency.



Improvement Type	Recommended	0		Construction by District		Construction by Contractor	
Improvement Type	Diameter	Diameter Quantity		Unit Cost	Total Cost	Cost Factor	Total Cost
Loma Court Fire Flow Improvement							
Replacement Pipeline	8	500	LF	\$230	\$115,000	\$332	\$166,000
Hydrant Replacement		1	EA	\$6,500	\$6,500	\$18,000	\$18,000
Service Connection Replacement		11	EA	\$2,000	\$22,000	\$5,000	\$55,000
Main Tie-ins		1	EA	\$2,500	\$2,500	\$10,000	\$10,000
	Const	ruction	Contir	ngency (25%)	\$36,500		\$62,300
			Cons	truction OPC	\$182,500		\$311,300
Engineering, Administration, and Permitting Costs (25%)							\$62,300
	Total OPC						\$370,000



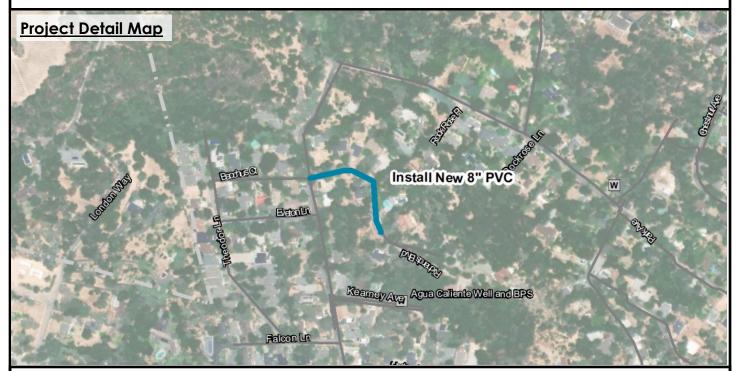
Project ID: P-25 Project Priority Level: 3

Description: Richards Blvd Fire Flow Improvement

Location: Richards Blvd.

Improvement Details: Replace existing 6-inch ACP and DIP water main with 8-inch PVC water main along Richards Blvd, replace four existing service connections, and one existing hydrant.

Justification: Addresses minor fire flow deficiency.



Improvement Type	Recommended	Quantity		Construction by District		Construction by Contractor					
	Diameter			Unit Cost	Total Cost	Cost Factor	Total Cost				
Richards Blvd. Fire Flow Improvement											
Replacement Pipeline	8	300	LF	\$230	\$69,000	\$332	\$99 <i>,</i> 600				
Hydrant Replacement		1	EA	\$6,500	\$6,500	\$18,000	\$18,000				
Service Connection Replacement		4	EA	\$2,000	\$8,000	\$5 <i>,</i> 000	\$20,000				
Main Tie-ins		2	EA	\$2,500	\$5,000	\$10,000	\$20,000				
Construction Contingency (25%)							\$39 <i>,</i> 400				
Construction OPC							\$197,000				
Engineering, Administration, and Permitting Costs (25%)					\$22,100		\$39,400				
Total OPC					\$130,000		\$240,000				



Project ID: P-27 Project Priority Level: 2

Description: SCWA Turnout Flow Meter Installation

Location: Various (Each SCWA Turnout PRV Station)

Improvement Details: Install flow meters at each of Turnout PRV Stations and integrate with SCADA system.

Justification: Provides District ability to verify SCWA billing and perform zonal demand analyses.

Improvement Type	Recommended	Quantity		Construction by District		Construction by Contractor				
	Diameter			Unit Cost	Total Cost	Cost Factor	Total Cost			
SCWA Turnout Flow Meter Installation										
Install Flow Meters at Turnouts		10	EA	\$48,000	\$480,000	\$65,000	\$650,000			
Construction Contingency (30%)							\$195,000			
Construction OPC							\$845,000			
Engineering, Administration, and Permitting Costs (30%)					\$144,000		\$195,000			
Total OPC					\$770,000		\$1,040,000			



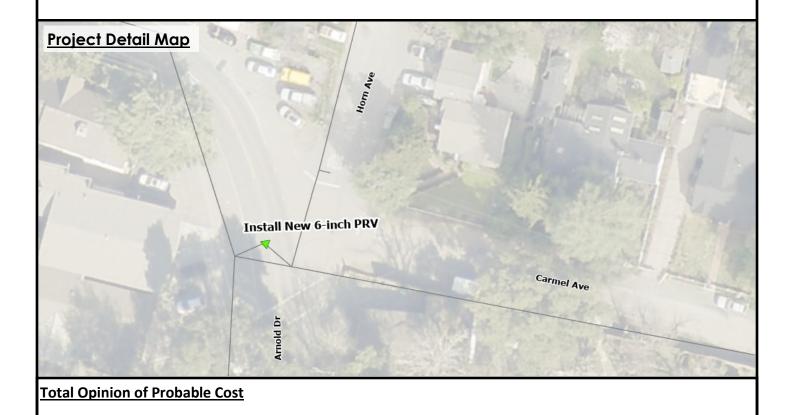
Project ID: P-28 Project Priority Level: 2

Description: District Metered Area 1

Location: the corner of Horn and Carmel Ave in Glen Ellen.

Improvement Details: Install new 6-inch PRV station with flow metering at the corner of Arnold Drive and Carmel Ave and create new pressure zone in Glen Ellen.

Justification: Reduces excessive pressures in this area.



Recommended **Construction by District Construction by Contractor** Quantity **Improvement Type Total Cost** Diameter Unit Cost **Cost Factor Total Cost** District Metered Area 1 (PZ-1H) **PRV** Installation \$90,000 \$90,000 \$120,000 \$120,000 LS 6 1 Main Tie-ins 2 \$2,500 \$5,000 \$10,000 \$20,000 --EΑ Flow Meter Install --1 LS \$48,000 \$48,000 \$65,000 \$65,000 Construction Contingency (30%) \$42,900 \$61,500 Construction OPC \$266,500 \$185,900 Engineering, Administration, and Permitting Costs (30%) \$42,900 \$61,500 Total OPC \$230,000 \$330,000



Project ID: P-29 Project Priority Level: 1

Description: District Metered Area 2

Location: Agua Caliente Knolls

<u>Improvement Details</u>: Install new 8-inch PRV station with flow metering at the intersection of Kearney Avenue and East Agua Caliente Road, running parallel to the existing zone separating closed valve, and new 12-inch PRV stations with flow metering (1) on West Agua Caliente Road east of the roundabout (2) on Highway 12 between Vailetti Drive and Sunnyside Avenue to create new pressure zone 1G in the Agua Caliente Knolls area.

Justification: Increase pressure to above 35 psi and increase fire flow availability for multiple hydrants in the Agua Caliente Knolls area to better meet system performance criteria.



Improvement Type	Recommended	Quantity -		Construction by District		Construction by Contractor				
	Diameter			Unit Cost	Total Cost	Cost Factor	Total Cost			
District Metered Area 2 (PZ-1G)										
PRV Installation	6	3	LS	\$90,000	\$270,000	\$120,000	\$360,000			
Main Tie-ins		2	EA	\$2,500	\$5,000	\$10,000	\$20,000			
Flow Meter Install		3	LS	\$48,000	\$144,000	\$65,000	\$195,000			
	\$125,700		\$172,500							
Construction OPC							\$747,500			
Engineering, Administration, and Permitting Costs (30%)							\$172,500			
Total OPC							\$920,000			



Project ID: P-30 Project Priority Level: 2

Description: District Metered Area 3

Location: Temelec

<u>Improvement Details</u>: Install new 6-inch PRV stations with flow metering on (1) Avenida Sebastiani between Via Colombard and Avenida Barbera and (2) on Arnold Drive between Mission Drive and Avenida Sebastiani, and close the valve on South Temelec Circle between Mission Drive and Herbazal Street to create new pressure zone 1I in the Temelec Area.

Justification: Reduce excessive pressures from above 100 psi down to 60 psi in the southern portions of PZ-1A.



Improvement Type	Recommended	Quantity		Construction by District		Construction by Contractor				
	Diameter			Unit Cost	Total Cost	Cost Factor	Total Cost			
District Metered Area 3 (PZ-11)										
PRV Installation	6	2	LS	\$90,000	\$180,000	\$120,000	\$240,000			
Main Tie-ins		2	EA	\$2,500	\$5,000	\$10,000	\$20,000			
Flow Meter Install		2	LS	\$48,000	\$96,000	\$65,000	\$130,000			
Construction Contingency (30%)							\$117,000			
Construction OPC							\$507,000			
Engineering, Administration, and Permitting Costs (30%)							\$117,000			
Total OPC							\$620,000			



Project ID: P-31 Project Priority Level: 1

Description: Arnold Drive and Agua Caliente Road Roundabout Improvement

Location: Intersection of Arnold Drive and Agua Caliente Road

Improvement Details: Replace existing 8-inch ACP water mains with new 12-inch PVC water mains and relocate the existing Hannah Lower PRV out of the center of the new roundabout.

<u>Justification</u>: This project has been identified as high priority due to the safety concerns with operating the Hannah Lower PRV in the center of a intersection with high traffic flow. This project could be combined with P-7 for efficiency.

